

# **SOME STUDIES ON RURAL WATER SUPPLY SCHEMES**

**A Thesis Submitted  
in Partial Fulfilment of the Requirements  
for the Degree of**

**MASTER OF TECHNOLOGY**

*By*

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*to the*

**DEPARTMENT OF CIVIL ENGINEERING  
INDIAN INSTITUTE OF TECHNOLOGY KANPUR  
MAY, 1980**



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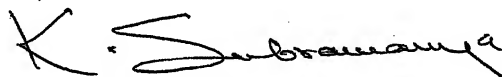
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## CERTIFICATE

This is to certify that the thesis 'Some Studies on Rural Water Supply Schemes' submitted by Mr. R.P. Yadav in partial fulfilment of the requirements for the degree of Master of Technology of the Indian Institute of Technology, Kanpur, is a record of bonafide research work carried out by him under my supervision and guidance. The work embodied in this thesis has not been submitted elsewhere for a degree.



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Dated: May, 1980



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## LIST OF SYMBOLS AND ABBREVIATIONS

## Symbols

C	Hazen Williams Roughness Coefficient
$\gamma$	Specific Weight
$\gamma_w$	Specific Weight of Water
S	Specific Gravity

## Abbreviations

WHA	World Health Assembly
LSGED	Local Self Government Engineering Department
IDA	International Development Authority
LIC	Life Insurance Corporation
RWSS	Rural Water Supply Scheme
RWS	Rural Water Supply
l.p.c.d.	Litres per capita per Day
A.C.	Asbestos Cement
P.V.C.	Polythene Polyvinyl Chloride
G.I.	Galvanized Iron
C.I.	Cast Iron
B.H.P.	Break Horse Power
UNICEF	United Nations International Children's Emergency Fund
NWSP	National Water Supply and Sanitation Programme
O.H.T.	Over Head Tank



RCC	Reinforced Cement Concrete
KL	Kilolitre
PSIG	Per Square Inch Gauge
MGD	Million Gallons Per Day
GPM	Gallons Per Minute
CFS	Cubic Feet per Second
l/s	Litres per Second
LP	Linear Programming
HGL	Hydraulic Grade Line



## ABSTRACT

The rural water supply schemes of U.P. Jal Nigam have been studied by taking five representative projects. The present studies of design criteria and the overall typical characteristics of these water supply schemes have been highlighted. The need for an efficient design and analysis of water distribution networks has been brought out.

A versatile water distribution network system analysis programme has been implemented at I.I.T. Kanpur DEC 1090 system. The five selected rural water supply schemes were analysed through the use of this versatile programme. The deficiencies in the existing schemes and the methods to modify them have been indicated.

A method for the optimum design of tree type water distribution network has been indicated.



## 1 INTRODUCTION

### 1.1 GENERAL

Water, like air, is essential to human survival, so it is a must for a welfare government to provide safe and potable drinking water in adequate quantity to its urban as well as rural population. In recent few years there is a progressive effort towards this.

The United Nations Water Conference held at Mar del Plata, Argentina, in March 1977 recommended that the decade (1980-1990) should be designated as the 'International Drinking Water Supply and Sanitation Decade'. The resolution WHA 30.33 concerning preparations for the decade (1980-1990) has been unanimously adopted by the thirtieth World Health Assembly in May 1977. According to this (i) Implementation of National Plans for drinking water supply and sanitation to all urban and rural communities should be increased, (ii) Specific targets should be set up by each country taking into consideration its sanitary, social and economic conditions<sup>20</sup>. The above is an important resolution and it is but natural that the Central and State Governments of India will give high priority for drinking water supply and sanitation programme, and provide necessary provisions in the annual plans so as to achieve these objectives.



According to the World Health Statistics Report, 1976, about 77 percent of the urban communities and 22 percent of the rural communities among the developing countries have been provided with adequate water supply upto 1975. Progress in urban water supply in India is comparable to that at global and South East Asia regional level, but it is rather poor in rural water supply systems. In India 60 percent of the urban community and 4 percent of the rural community was served by water supply facility in 1970 and 79 percent and 18 percent in 1975 respectively, as indicated in Table 1.1.

TABLE 1.1 WATER SUPPLY POSITION IN INDIA

Community	Population served (percent)		Population to be served (Target) (percent)	
	1970	1975	1980	1990
Urban	60	79	98	100
Rural	4	18	32	100

As per 1971 Census the country had an urban population of 1090.94 million spread in 3,119 towns and rural population of 438.58 million in 5,75,936 villages and hamlets. The population in 3,18,633 villages (55.32 percent) is less than



500. An additional 1,32,990 villages have population in the range of 500-999 making a total of 4,51,626 (78.41 percent). These villages are scattered in the countrysides, some of which are inaccessible during part of the year whereas some have no reliable source nearby. A total of 1,52,475 villages came under the category of scarcity or problem villages, which pose a challenge to those involved in water supply programme.

Water with high chemical ingredients such as fluorides, chloride, iron and manganese have been found in about 25000 villages with a population of approximately 21.93 million. In addition water borne diseases such as dysentery, gastro enteritis, infectious hepatitis, enteric fevers and poliomyelites are prevalent in urban and rural areas. The death rate due to these diseases have been found to be 9.6 and 16.5 per thousand persons in urban and rural communities respectively. This reflects an urgent need of safe water supply and other public health facilities, especially in rural areas.

Out of the total annual precipitation of 400 million hectare meter, only about 105 million hectare meter is estimated to be ultimately utilizable containing of 70 million hectare meter of surface water and 35 million hectare meter of ground water. The surface water is not safe as it gets



polluted due to increased industrialization. In ground water there is presence of chemical ingredients. So, there is a need of some technological solution for safe water supply. These are the reasons why India Government is giving more emphasis on rural water supply schemes, from Fourth Five Year Plan (1969-74). The accelerated water supply programme gave a further boost to rural water supply. About 64,000 out of 5,75,936 villages and 1890 out of 3,119 towns have been provided with water supply by 1978. The outlay on rural water supply was 0.8 percent of the total plan during the first plan which increased to 1.5 percent in the fifth plan. The annual per capita expenditure on rural water supply and sanitation for the unserved population of the entire country was increased from Re 1.00 from the fourth plan to Rs. 2.4 in the fifth plan (1974-1978).

Several financing institutions like the Life Insurance Corporation of India and Banks have been providing part of funds required for water supply schemes. Funding from international and bilateral agencies are available now.

#### 1.2 U.P. JAL NIGAM

Previously supplying of water to a community was the responsibility of the municipal authorities. But when the work grew in complexity and volume, need for the establishment



of a separate organisation was felt and in 1927, the U.P. Public Health Engineering Department was created. It was later re-named the Local Self Government Engineering Department (LSGED) which is now fully merged in the U.P. Jal Nigam. Further, State Government gave more emphasis on water supply and sanitation which required large initial investment. So, in the seventies the World Bank was approached. But to give financial assistance it wanted a central organisation with statutory powers to be established first, through which loans to the local bodies could be channelized. Due to this the State Government created an organisation named U.P. Jal Nigam on June 18, 1975 by an act of U.P. Legislature. Now it is a dynamic organisation of U.P. Government, which is working under the financial assistance mainly of International Development Authority, (IDA), a subsidiary of the World Bank for soft lending to member nations of United Nations Organisation for development programmes; the Dutch Government and the Life Insurance Corporation of India (LIC)<sup>7</sup>.

It consists of eight Jal Sansthan, three Regional Jal Sansthan namely Garhwal, Kumaon and Bundelkhand and five local Jal Sansthan for KAVAI towns ie. Kanpur, Agra, Varanasi, Allahabad and Lucknow to give proper water supply and sewerage facilities.



During the five Five-Year Plans only 6,670 villages were brought under the piped water supply. Out of the 1,12,561 revenue villages in the state 35,506 villages suffer from scarcity of potable water. Similarly out of the 619 urban localities barely 376 localities had piped water supply. Till the end of the Fifth Five Year Plan only 39 urban localities were provided the sewerage facility. The Nigam provided water to 1,917 villages during the year 1978-79. This compares very favourably with the 6,670 villages which were provided this facility during the last five Five Year Plans, i.e. upto 1978, and that. During the year 1977-78, drinking water was provided to 1,200 villages.

During the Sixth Five Year Plan period 1978-83 it has been envisaged to provide piped water supply to all the 619 urban local bodies of the state as well as to provide underground sewerage system in 39 more towns. Strengthening of water supply has been proposed in 83 towns and sewerage reorganisation in eight towns. A target covering 14,200 more villages has been set for rural water supply programme. To boost the progress of rural water supply schemes, the Central Government has launched a crash programme under the title, 'Accelerated Rural Water Supply Programme' and it is likely that a sum of Rs. 26 crores will be made available for this, besides the plan allocations. This will be utilised



for provision of safe drinking water supply facilities to about 2,250 additional villages.

### 1.3 DESIGN CRITERIA

The design criteria for the RWSS adopted by U.P. Jal Nigam with recent guidelines is as following:-

#### Design Period

The design period should be 30 years.

#### Population

The rural design population should not be estimated by the previous criterion of increasing the present population by 50 percent except the villages bordering the towns where the percentage increase in the design population may be more.

Any one of the following methods is to be adopted to calculate the design population.

- i) Arithmetical mean method
- ii) Geometrical increase method
- iii) Incremental increase method
- iv) Decrease in percentage increase method
- v) Graphical method
  - a) an ordinary plot method
  - b) a semi-log plot method
- vi) Comparative method



### Floating Population

The equivalent population for feeding the institution, market and mela may be considered as 20 percent of the total population.

### Rate of Water Supply

- (i) 70 l.p.c.d. for villages where individual design population is less than 4,000 and where private house connections are expected.
- (ii) 50 l.p.c.d. for small villages where no or few house connections are expected.
- (iii) 25 l.p.c.d. where adequate source is not available. But the schemes should be framed on 50 l.p.c.d. so that except for 2 to 3 months of the summer adequate quantity of water is available to the beneficiaries.
- (iv) 90 l.p.c.d. for villages where individual design population is more than 4,000.

### Source Discharge

The discharges of the source should normally be measured for three consecutive years of the driest season and the least discharge should be adopted. In the case if the scheme is to be prepared urgently, the one year driest discharge should be double than the ultimate requirement.



In special circumstances where there is an acute scarcity of drinking water the scheme can be framed if the discharge of the source can feed at a rate of 25 l.p.c.d.

#### Service Reservoir

Minimum storage however should not be less than 10 l.p.c.d. or 1/6th of the days requirement whichever is more.

#### Peak Factor

For KAVAL towns	2.0
For Urban towns	2.5
For Rural areas	2.5
For Industrial areas	1.0

These peak factors are applicable on maximum day per capita rates adopted.

#### Distribution System

The distribution network should be designed for peak flow of peak factor times the average demand.

The minimum size of a distribution main should be kept according to the size of the town as per following criteria.

Design Population	Minimum Size of Distribution Main
Upto 5,000	50 mm
5001 to 50,000	80 mm
50,001 to 5,00,000	100 mm
above 5,00,000	125 mm



In hills the minimum size shall be 25 mm.

For towns of population above one lac wherever the main road is more than 20 m in width, mains will be laid on both sides so that the road is not cut for service connections later on.

### Pipes

Medium quality of G.I. pipes should be used for water supply schemes of hilly areas.

In plains P.V.C. pipes may be adopted upto a dia of 100 mm. A.C. pipes should be used for bigger sizes. Presently upto 150 mm P.V.C., 200 to 600 mm A.C. and above 600 mm C.I. pipes are being used.

### Terminal Pressures

Terminal pressures shall be provided as listed in the following tabular shape.

Design Population of the town(in lacs)	Building type	Terminal Pressure (m)
Upto 0.20	Single storied	7
	Double storied	12
0.20 to 0.50		
0.50 to 1.00	No consideration of the height of building	12
1.00 to 5.00		
5.00 to 10.00	No consideration of the height of building	15
above 10.00		



Terminal pressure for villages shall be 7.0 meters. Minimum terminal pressure of 6 m for single storied buildings with market and a few house connections and 8 m for double storied building with a large number of house connections were adopted previously by Nigam.

#### Pipe Lines

Sluice valve/wheel valve may be provided at a spacing of 21 cms (for sizes greater than 150 mm, the spacing may be reduced as required).

Terminal pressure - Minimum 6 m

Design of distribution system are to be done for each and every village. Station pressure must be duly considered for deciding classes of pipes. Break pressure tables are to be provided as required.

Stand posts considering initial population (upper limit)

- (i) One for about 150 persons in hilly areas
- (ii) One for about 250 persons in plains.

There should be one stand post atleast for weaker section such as Harijan and Tribal community.

Air valve should be provided on summits.

Tank type stand posts, instead of single tap pillar type previously used.

- (i) In such villages where no private connections are



expected in the beginning, tank type stand posts with capacities of 2000, 3000, 5000 litres may be provided, keeping in view 1/2 day's requirement in the initial stages.

- (ii) The distribution system even in these villages will be designed for 6 meters, minimum terminal pressure to provide for house connections in future.

#### Rising Main

The economical size of rising main should be calculated as per departmental tabular procedure taking electricity tariff rule for the cost of every corporation economics of laying a size main throughout the design period to that of duplicating after 15 years be worked.

For tube wells and other pumping schemes requiring short length of rising main (say not exceeding 200 m), it would be desirable to use C.I. pipes. At other place A.C. pipe tested upto adequate pressure may be considered.

#### Consumption

The annual average supply of water is expected to be about 75 percent. The consumption of chemicals, electric or diesel should be calculated on this pattern. Minimum of two tube wells may be provided. The pumping hours of the tube well may generally be taken 16 hrs at the ultimate period of the scheme.



### Hydraulic Gradient

To design the distribution main, such hydraulic gradient may be adopted for different type of pipe materials as to provide an economical design. For this purpose the hydraulic gradient arrived at for economic design of rising main shall be adopted. Generally the hydraulic gradient for P.V.C. and A.C. pressure pipes shall be 3 to 4 per thousand and for C.I. and steel pipes 5 to 6 per thousand. However the gradient may vary according to the minimum size of the distribution main as recommended in para 'Distribution System'.

### Size of Pump House

BHP	Size
10.0	8' x 10' x 12'
12.5 to 40.0	12' x 10' x 12'

The chloronome house is to be constructed jointly with pump house as per type design.

### Accommodation of the Staff

- (i) For pump operator - one single room quarter with box room.
- (ii) For chawkidar - one single room quarter with one box room.

Provision for more buildings is to be made after



obtaining Chief Engineer's instruction. For gravity schemes where provision of part-time staff for maintenance purposes is considered adequate, number of buildings should be provided.

#### List of Various Categories of Pipes and Their Working Pressures

##### (i) A.C. Pipes

Type of Pipe	Working Pressure
Class I	2.5 kg/cm <sup>2</sup>
Class II	5.0 kg/cm <sup>2</sup>
Class III	7.5 kg/cm <sup>2</sup>

##### (ii) G.I. Pipes

Size (mm)	Pressure (kg/cm <sup>2</sup> )		
	Light	Medium	Heavy
6 to 25	10.5	21.0	24.6
32 to 40	8.8	17.6	21.1
50 to 80	7.0	14.0	17.6
80 to 100	5.6	10.5	14.0
125	-	10.5	14.0
150	-	8.8	10.5

##### (iii) P.V.C. Pipes

These are available in 4 , 6 and 20 kg/cm<sup>2</sup> working pressures, M/s Wavin India Ltd. have, however, offered pipes capable of withstanding a working pressure of 5 kg/cm<sup>2</sup> against 4 kg/cm<sup>2</sup> pipes.



## (iv) C.I. S/S Pipes

Class	Pressure(kg/cm <sup>2</sup> )
LA	6
A	9
B	12

## Design Formula

Hazen and Williams formula is to be used for the design of the distribution system.

## 1.4 FINANCE

Considerable emphasis is being given to the RWSS. as laid in the 'International Drinking Water and Sanitation Programme' to make all the villages of the countries provided with the safe drinking water by 1990. Our national Government is also taking keen interest in this field to fulfill the target. But to do this our developing country needs much financial assistance. So this assistance is being taken from International and National financing concerns. For the U.P. Jal Nigam the finance pattern has two ways - Plan and Non-plan. Under plan, the finance is given by IDA and Non-IDA procedures. In IDA procedure the finance comes 50 percent from IDA, an affiliate of the World Bank and 50 percent from the LIC (India). Under Non-IDA, the finance is provided by Central and State Governments through granting and loaning ways, according to the



financial situation of the area concerned, and by the LIC and other Banks through loaning, which is to be returned in few years by the Department. Government gives 100 percent grant to the scheme whose public finance condition is poor and there is no proper source of return of money and 75 percent grant and 25 percent loan or 50 percent grant and 50 percent loan, to the schemes whose area is prosperous. Under the non-plan deposit work comes. If Department contracts some private concern's scheme, the Department needs full cost of the scheme in advance deposited and work will be started after full payment to the Department. To accelerate the National Water Supply and Sanitation Programme (Rural), the financial assistance is to be needed urgently. For this at this time UNICEF and Dutch Government are giving finance.

#### 1.5 NEED FOR AN OPTIMAL DESIGN

The distribution system of a water supply scheme has a major part of the cost incurred on the scheme. So, it should be designed economically. There is no proper and accurate method to design the network system in an optimal manner. But there are more works on network analysis. By this analysis we can optimize the network by changing the diameters of the pipes. Nowadays we have computer facilities available generally in our country, so that through computers we can analyse even a big network within seconds by using an efficient computer



Programme. The RWSS where finance is a main problem, can benefit considerably through efficient, optimal designs. In the present study an analysis of few RWS schemes under implementation of U.P. Jal Nigam have been made by a modified and efficient computer programme to bring out the efficiency and advantages of computer use in such large investment projects.



## 2 PRESENT STUDY

### 2.1 GENERAL

The topic of RWS in U.P., for which not only our State or Central Government is taking interest but some other Foreign Governments and International Organisations are giving help, has been selected for the study. The Chief Engineer (Project and Design) and one Research Executive Engineer of U.P. Jal Nigam provided the necessary background and details about the RWS schemes. Reports of three RWS schemes of District Allahabad were made available. Also the Executive Engineer, IInd T.C.D. Kanpur provided valuable informations and details of two RWS schemes of District Kanpur. The details of the various schemes are in the next sections.

### 2.2 SALIENT FEATURES

#### A. Girdkot, Zone A, Group of Villages RWSS<sup>14</sup>.

Name of programme	: N.W.S.P. (Rural)
Name of Local Body	: District Board, Allahabad
Number of Villages Covered	: 16
Population	: Present: 7,476 (1971) Designed: 11,214 (2001)
Rate of W/S	: 70 l.p.c.d.
Source of W/S	: Tube well
Nature of Treatment	: Pressure feed type chlorination



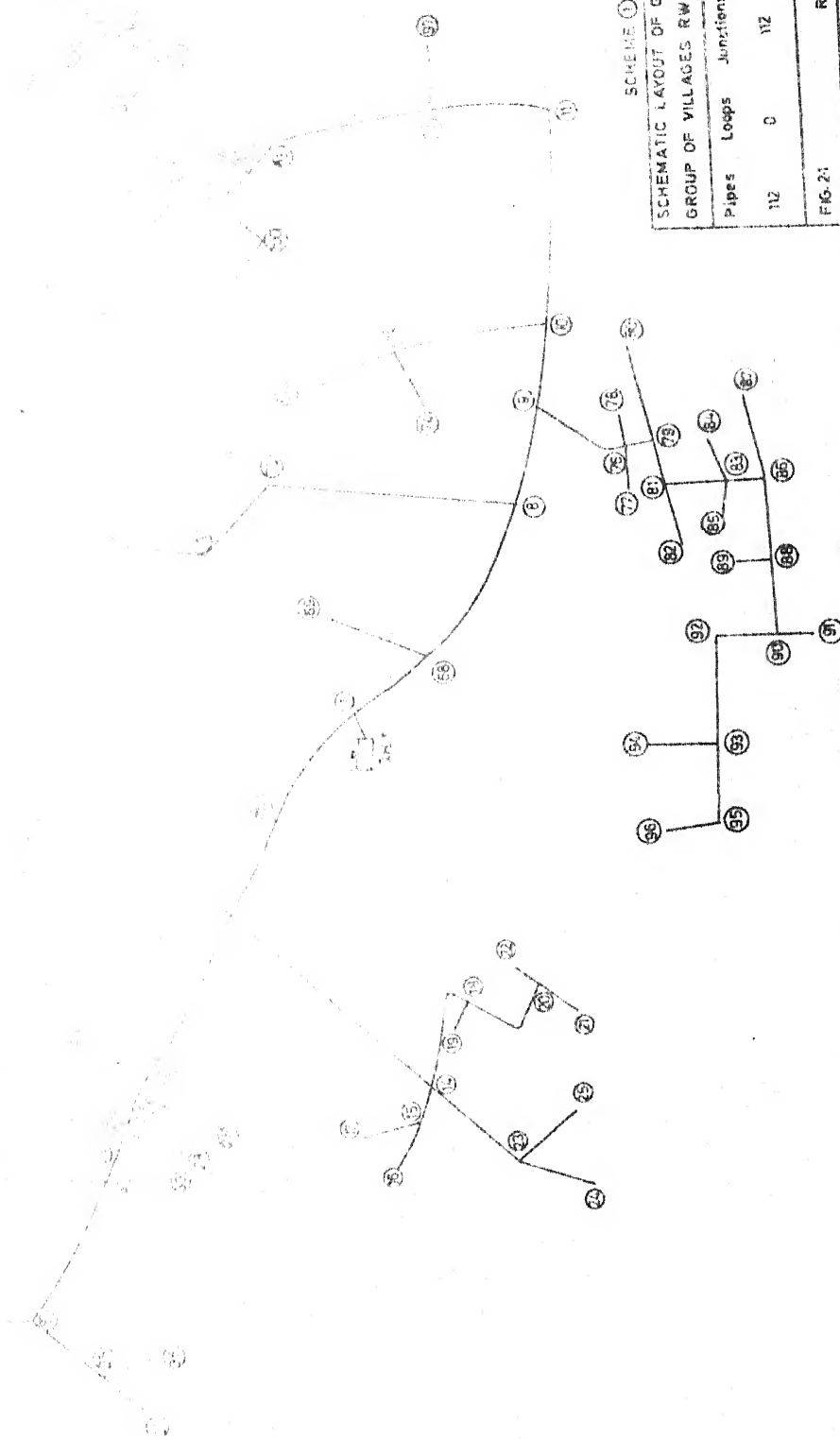
SCHEME ①

SCHEMATIC LAYOUT OF GIRDKOT, ZONE A,  
GROUP OF VILLAGES RWSS, ALLAHABAD

Pipes	Loops	Junctions	Fixed grade node
112	0	112	1

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FIG. 21





Conveyance : A.C. and P.V.C. pipes  
 Service Storage : One O.H.T. of  
                   Capacity : 200 KL (based on 1/4<sup>th</sup>  
                                   of the ultimate  
                                   daily requirement  
                                   of water)  
                   Staging : 15 m  
                   Material : RCC  
                           of cons-  
                           truction  
 Distribution system : Peak factor : 2.4  
                           Minimum size: 25 mm (internal)  
                           Minimum terminal pressure: 7 m  
                           Kind, size and class of pipe

Kind of pipe	Size, $\phi$ (mm)	Class	C
A.C.	125 to 250	10	130.00
P.V.C.	25 to 100	4 to 10 kg/cm <sup>2</sup>	140.00

Estimated Cost : 9.00 lacs  
 Pumping plant : Plant capacity: 25 BHP  
                   Pumping Hours : 10  
 Draw off rate of balancing reservoir : 16 Hours (standard)

This scheme comes in Handia Tehsil of Allahabad District.

The area under Handia Tehsil is such that there is no river or any other natural source to get drinking water in abundant quantity. It is very difficult to get water. In summers the draught condition is a regular feature. The ground water



condition is good here, so this water is to be utilized for drinking. This rural water supply scheme has been drawn up for the villages which are draught affected and the villages which come in the alignment of pipe lines for draught stricken villages.

Taking the design period 30 years, the designed population at the end of the design period i.e. in the year 2001 has been taken as 11,214 assuming 50 percent increase over the 1971 population taking into account the future growth and development.

To make the scheme economical it was divided into zones as follows:

Zone	Number of villages	Population designed
A	16	11,214
B	21	11,814
C	21	14,590

B. Ketehra, Zone A, Group of Villages RWSS<sup>15</sup>

Name of the programme : NWSP (Rural)

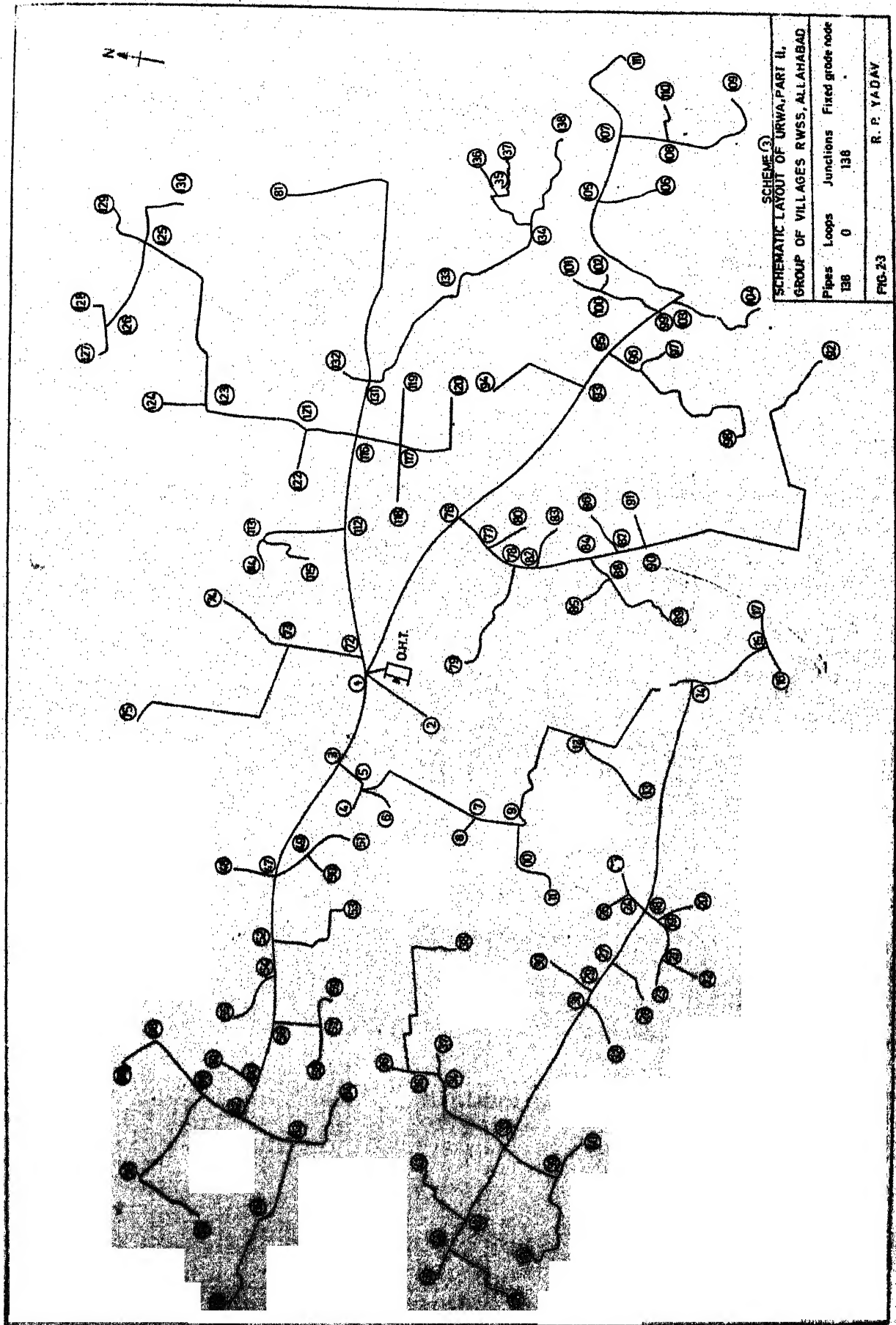
Name of the Local body : District Board, Allahabad

Number of villages covered : 13

Population : Present:7,123 (1971)  
Designed:10,693 (2001)

Rate of W/S : 68.1 l.p.c.d.







Nature of source : Tube well  
 Source of Development : Pump House  
 Nature of treatment : Pressure feed type chlorination  
 Conveyance : A.C. and P.V.C. pipes  
 Service storage : One RCC O.H.T. of  
    Capacity: 120 KL (based on 1/6th of  
    the ultimate daily  
    requirement of water)  
    Staging : 17 m  
 Distribution system : Peak demand factor: 2.4  
    Minimum terminal head: 7.0 m  
    Minimum size : 25 mm  
    kind, size and class of pipe

Kinds of pipes	Size, Ø(mm)	Class	C
A.C.	125 to 250	II	130.00
P.V.C.	25 to 100	-	140.00
G.I.	15 to 20	Light quality	-

Estimated cost : 9.0 lacs  
 Pumping plant : Plant capacity: 20 BHP  
    Pumping Hours: 10  
 Draw off rate of balancing reservoir : 16 Hours (standard)

This scheme also comes in Handia Tehsil of District Allahabad. Due to absence of river or any other natural source the ground water is to be supplied for drinking to the draught



stricken, Ketehra group of villages, the condition of which is good.

As per 1961 Census, the population of all the 52 villages covered by this scheme was 19,643. The estimate of this scheme was originally prepared by scarcity division, Mirzapur in the year 1972-73 and the population of 1971 Census was taken as the initial population by increasing the population of 1961 Census by 25 percent which comes to be 24,541. This was assumed that over a period of 30 years further increase in population would be 50 percent keeping a margin for family planning measures and migration of rural population to urban areas, including floating population due to occurrence of cattle fair and religious festivals.

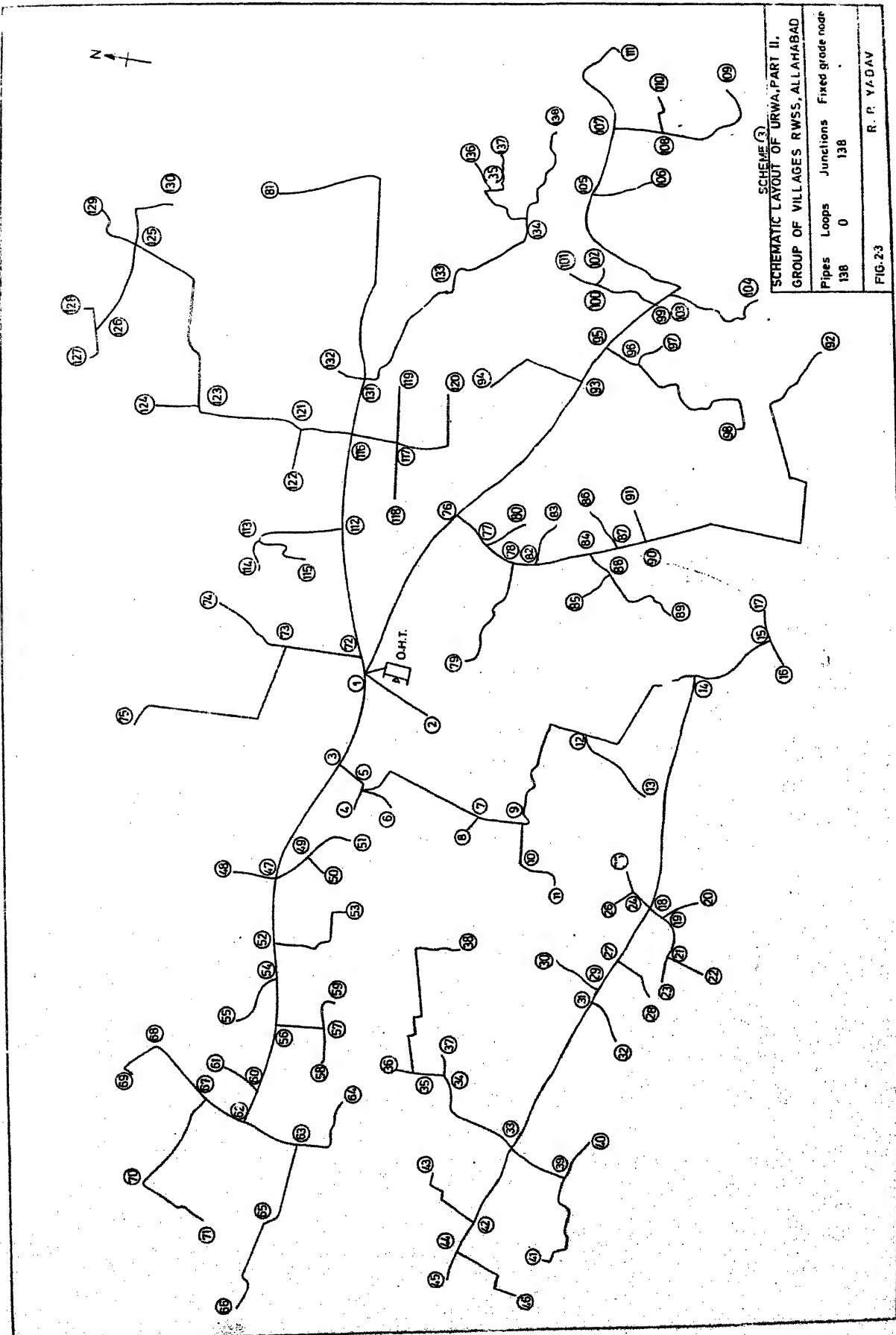
To make the scheme economical it was divided into three zones as follows:

Zone	Number of villages	Population designed
A	13	10,693
B	20	16,593
C	19	9,533

#### C. Urwa, Part II, Group of Villages RWSS<sup>16</sup>

Name of programme : Dutch programme  
 Name of local body : District Board, Allahabad  
 Number of villages covered : 23







Population : Census, 1971 : 16,836  
 Present, 1981 : 20,246  
 Designed, 2011 : 30,369

Rate of W/S : 70 l.p.c.d. for population < 4,000  
 90 l.p.c.d. for population > 4,000

Source of W/S : Tube well

Nature of treatment : Chlorination by differential pressure feed type chlorinating plant

Conveyance : Forced main of C.I. (class, LA, C=100.00) S/S pipe of

Ø(mm)	Length(m)
200	300
300	50

Service storage : One O.H.T. of R.C.C. having  
 Capacity: 650 KL (based on 1/4th of the ultimate daily requirement of water)  
 Staging : 14m

Distribution system : Peak demand factor : 2.4  
 Terminal head: 6m for single storied building with few house connections  
 8m for double storied building  
 Minimum size of pipe: 25 mm



## Kind, size and class of pipes

Kind	Size, $\phi$ (mm)	Class	C
A.C.	200 to 250	10	130.00
P.V.C.	25	10 kg/cm <sup>2</sup>	140.00
	32 to 40	6 kg/cm <sup>2</sup>	140.00
	50 to 150	4 kg/cm <sup>2</sup>	140.00

Appurtenances	:	Sluice valve	:	52 nos.
		Wheel valve	:	4 nos.
		Air valve single ball screwed down type 20 mm	:	12 nos.
		Fire hydrant	:	15 nos.
		Single tap type Stand post	:	42 nos. (30 percent for scheduled cast localities)
Average dose of chlorination	:	0.5 PPM		
Estimated cost	:	39.23 lacs		

This scheme falls under Tehsil Meja, District Allahabad. Scarcity of drinking water is a regular feature in summer in this area. To face the draught conditions in every summer of the area a permanent piped water supply has been proposed. The railway line from Allahabad to Mirzapur goes across through this village group. In the southern side of the railway line there is URWA, Part I, group of villages RWSS which has already been implemented. In the north side of the same this scheme



is to be implemented.

The base year for the design period is 1981. As per Census figures the growth of population in the district is 20.4 percent for 1961 to 1971 decade; so to arrive at the population for 1981, the 1971 population has been increased by 20.4 percent. The ultimate population has been calculated for the year 2011 anticipating an increase of 50 percent. Over the 1981 population.

Two centrally located tube wells with pumping plants designed for discharge of 2000 lpm, at a head of 40 meters with 25 HP motor running 6.5 hours each in the beginning, 8 hours in the middle and 9.5 hours at the end of the design period shall be sufficient to cope with the need of scheme.

#### D. Malasa Group of Villages RWSS<sup>17</sup>

Name of the programme : NWSP (Rural)

Name of the local body : District Board, Kanpur

Number of villages covered : 15

Population : Present : 15,400 (1978)  
Designed: 23,100 (2008)

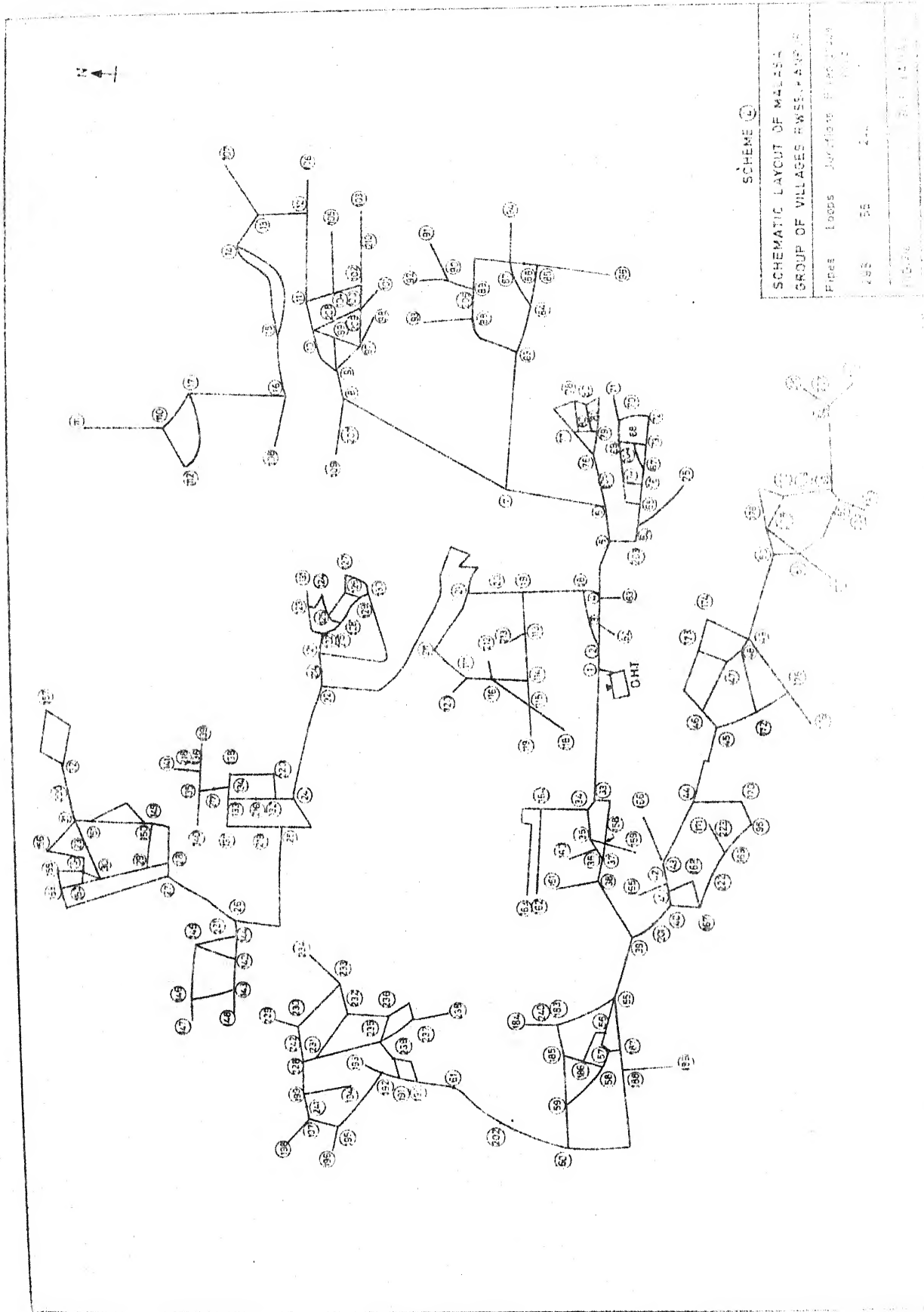
Rate of W/S : 70 l.p.c.d.

Nature of source : Tube well

Source of Development : Pump house

Nature of Treatment : Gaseous injector type chlorination using chlorine gas







- Conveyance : Forced main working on maximum pressure of 60 m water head of C.I. (class LA) having 200 mm internal dia and 330 m length.
- Service storage : One O.H.T. of RCC of  
Capacity : 500 KL (based a 1/4th of the ultimate daily requirement of water)  
Staging : 20 m
- Appurtenances : Sluice valves : 40 nos.  
Wheel valves : 35 nos.  
Air valves : 5 nos.  
Scour valves : 5 nos  
Public stand posts  
Proposed single tap  
pillar type : 75 nos. (18 nos. are for weaker section).
- Distribution system : Peak demand factor : 2.4  
Terminal' pressure : Min. 6m head  
Minimum size of pipe : 25 mm  
Kind, size and class of pipes
- | Kind   | Size, Ø(mm) | Class | C      |
|--------|-------------|-------|--------|
| A.C.   | 125 to 250  | II    | 130.00 |
| P.V.C. | 25 to 100   | -     | 140.00 |
- Dose of chlorination : Maximum : 2.0 PPM  
Average : 0.5 PPM  
Range : 0.1 to 2.0 PPM
- Estimated capital cost : 25.48 lac cs



Pumping plant : Two electrically driven oil lubricated vertical base hole turbine pumping plants

Plant capacity : 35 BHP

Pumping Hours : 16

Draw off rate of balancing reservoir : 16 hrs (standard)

Number of tubewells : 2 (one standbye)

The villages draw their daily requirement of water for drinking and domestic purposes from open wells and hand pumps which are inadequate in number and are liable to contamination. The available water of the wells in some of the villages is brackish as well as unhygienic which is not advisable for human use. Besides this the ground water level in this area is considerably low varying from 19 to 25 meters below ground level. So, piped water supply becomes essential in this area.

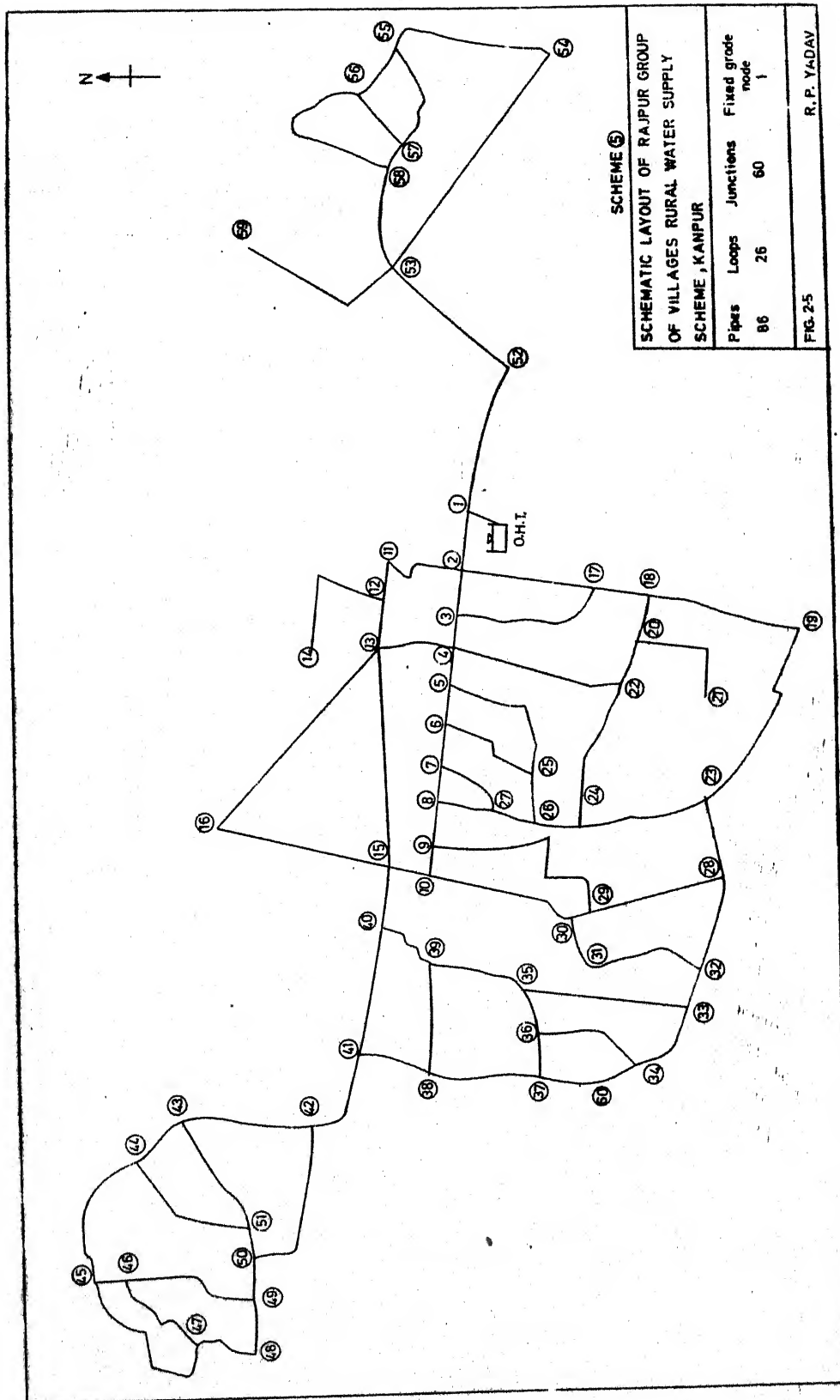
The 1971 Census population of the proposed group of villages is 13,883. The population in the beginning of the design period (1978) works out to be 15,400 by increasing the 1971 Census population by 12 percent. The design population after a period of 30 years (2008) works out to be 23,100 after increasing the initial population by 50 percent.

#### E. Rajpur Group of Villages RWSS<sup>18</sup>

Name of the programme : NWSP (Rural)

Name of local body : District Board, Kanpur







Number of villages covered:	3												
Population	: Present: 6,439 (1976) Designed:10,000(2006)												
Rate of W/S	: 70 l.p.c.d. for population <4,000 90 l.p.c.d. for population >4,000												
Nature of source	: Tube well												
Source of Development	: Pump house												
Nature of treatment	: Pressure feed type chlorination												
Conveyance	: Forced main of C.I.(class LA) having size $\phi$ (mm) length(m) 125 400 150 30												
Service storage	: One RCC O.H.T. of Capacity: 135 KL(based on 1/6th of the ultimate daily requirement of water) Staging : 12 m												
Distribution system	: Peak demand factor : 2.4 Minimum terminal pressure: 6 m Minimum size: 32 mm <table><tr><td>Kind</td><td>size <math>\phi</math>(mm)</td><td>Class</td><td>C</td></tr><tr><td>A.C.</td><td>65 to 200</td><td>II</td><td>130.00</td></tr><tr><td>P.V.C.</td><td>32 to 50</td><td>-</td><td>140.00</td></tr></table>	Kind	size $\phi$ (mm)	Class	C	A.C.	65 to 200	II	130.00	P.V.C.	32 to 50	-	140.00
Kind	size $\phi$ (mm)	Class	C										
A.C.	65 to 200	II	130.00										
P.V.C.	32 to 50	-	140.00										
Appurtenances	: Sluice valve : 15 nos. Air valve : 2 nos. Fire hydrant : 4 nos.												







The pressure heads at the terminal point are much more than that specified in design criteria. Some schemes were deficient and the pressure heads at the terminal points are much less than the specified, i.e. Girdkot, Urwa and Malasa. So, keeping in mind the minimum requirement of the pressure heads at the terminal points, the diameters of some pipes were changed according to the condition, to get more pressure head the diameter was increased, and final costs of the schemes were calculated. Table 2.1 shows the details of costs and savings.

TABLE 2.1: COST AND SAVING DETAILS OF THE SCHEMES

Schemes	Initial Cost (Rs.)*	Final Cost (Rs.)*	Saving (Percent)
A	3.42	3.44	-0.58
B	3.12	3.11	+0.32
C	15.00	15.26	-1.73
D	9.13	9.39	-2.85
E	1.78	1.57	+11.80

\* In lacs

#### 2.4 COMPARATIVE STUDY



TABLE 2.2 : COMPREHENSIVE ABSTRACT OF COST

Items	Schemes					
	A.1**	B.1***	C	D	E	
1	2	3	4	5	6	
Distribution system	14,47,600	15,12,000	19,30,111	11,80,000	2,25,000	
Over head tank	3,70,000	2,96,000	3,20,000	2,32,000	80,000	
Pumping plants	1,79,000	1,78,862*	1,34,000	1,40,000	40,000	
Pump House and chlorinating room	33,000	29,000	22,000	14,000	10,000	
Staff building	1,31,600	1,68,000 <sup>+</sup>	87,000	22,500	-	
Rising main	14,600	14,000	58,000	48,000	22,800	
Boundary wall, approach road and gate	47,200	-	20,000	7,500	1,08,000	
Electric transmission line	48,100	55,300	2,42,500	90,000	20,000	
Chlorinating plant	9,000	-	6,000	14,000	4,000	
Tubular shed	20,000	20,000	3,600	5,400	2,400	
Purchase of special T and P	71,000	71,000	29,000	15,000	-	

Table contd.....



Table 2.2 contd....

1	2	3	4	5	6
Extra cartage of material	5,000	-	-	8,500	4,000
Land acquisition	-	5,000	7,500	-	-
Water meters	-	-	33,060	29,600	10,000
Internal electrification	-	-	-	4,000	-
Provision of work required for diverting the surplus water for agricultural use	-	-	-	5,900	-
	23,76,100	23,49,162	28,92,771	18,16,400	5,26,200
Total (Rs. in lacs rounded off)	23.76	23.49	28.93	18.16	5.26

\* Cost includes for pumping plants plus chlorinating plant

+ Cost includes for staff building plus boundary wall, approach road and gate

\*\* Girdkot group of villages RWSS, Allahabad

\*\*\* Kethra group of villages RWSS, Allahabad



TABLE 2.3 : COSTS OF OVER HEAD TANKS

Sl. No.	Schemes	Sta- ging (m)	Capa- city (KL)	Cost (Rs.)	Year	Cost esti- mated in 1980 (Rs. in lacs)
1.	A	15	200	1,23,333	1978	1.36
2.	B	17	120	86,500	1978	0.95
3.	C	14	500	3,20,000	1979	3.36
4.	D	20	500	2,32,000	1976	2.82
5.	E	12	135	80,000	1975	1.02

Average cost per 100 KL = Rs. 0.654 lacs



TABLE 2.4: ECONOMICS

Items	Schemes					Average
	A.1*	B.1**	C	D	E	
	1971 (2001)	1971 (2001)	1981 (1996) (2011) <sup>x</sup>	1978 (2001) (2008) <sup>x</sup>	1976 (2006) -	2001
Total population	25,069 (37,618)	24,541 (36,819)	20,246 (25,307) (30,369) <sup>x</sup>	15,400 (21,520) (23,100) <sup>x</sup>	6,439 (10,000) -	
Total capital cost (Rs.)***	30.00 (30.00)	29.95 (29.95)	29.23 (29.23) (29.23) <sup>x</sup>	25.48 (25.48) (25.48) <sup>x</sup>	7.05 (7.05) -	
Total annual maintenance cost (Rs.)	86,000 (1,33,800)	78,000 (1,13,000)	86,000 (1,07,900) (1,29,800) <sup>x</sup>	1,45,000 (1,60,000) (1,11,820) <sup>x</sup>	35,700 (27,200) -	
Cost of scheme per capita (Rs.)	119.66 (79.75)	122.03 (81.34)	194.00 (155.01) (129.00) <sup>x</sup>	165.45 (118.40) 110.00	110.00 (71.00) -	101.10
Maintenance cost per capita (Rs.)	3.43 (3.56)	3.18 (3.07)	4.25 (4.26) (4.27) <sup>x</sup>	9.45 (7.43) (4.84) <sup>x</sup>	5.54 (2.72) -	4.21
Water cost per KL (Rs.)	0.13 (0.14)	0.13 (0.13)	0.21 (0.21) (0.21) <sup>x</sup>	0.37 (0.29) (0.19) <sup>x</sup>	0.22 (0.11) -	0.18
Total cost of scheme/1000 of population (2001)=Rs.1.05***						

\* Girdkot group of villages RWSS, Allahabad

\*\* Ketehra group of villages RWSS, Allahabad

\*\*\* In lacs



## 2.5 TYPICAL FEATURES OF A RWSS

According to the comparative study of the economics of the five U.P. Rural Water Supply schemes we get an average total of expenditure incurred on scheme on per capita basis. Per capita average total of expenditure on scheme will be equal to the sum of average per capita capital cost of the scheme and average per capita maintenance cost. The total for the year 2001 comes as Rs. 105.31. This per capita cost of the scheme is helpful for one to know the cost of the scheme, when population is known. One can easily get a total cost of the RWSS in a particular year, when the population to be served is known for that year, allowing a provision of appropriate annual compound interest (say 5 percent).

By the comparative study of over head tanks for all RWS schemes, the costs of the over head tanks for the year 1980 have been calculated by giving a provision of 5 percent annual compound interest. We have got an average total expenditure on over head tank on/100 KL basis, as Rs. 65,400.00. So, we can easily estimate the expenditure incurred on over head tank in an particular year of construction, if we know the capacity of over head tank.



It has been found from the present study that the Over Head Tank for each RWSS was designed and constructed separately. It incurs extra expenditure for its design for each scheme. It also creates difficulty in construction due to changes in each O.H.T.'s, so the working efficiency for the staff for construction becomes low giving more expenditure on scheme. Therefore, it is a need for standardization of O.H.T.'s. There can be a few O.H.T.'s. When we will go to design the distribution system, we can choose a standard design of O.H.T. of standard capacity to suit the scheme.

From the analysis of the network system of the schemes, it has been found that the required terminal pressure heads at some terminal points were not satisfied. There is no accurate method of the analysis in the Department. Manual analysis is very difficult. So, there is a need of distribution analysis by computer. With this we can modify the distribution system to get a nearly optimized network.

Also there is a need of standard features for staff buildings, approach roads and stores etc.



### 3 ANALYSIS OF WATER DISTRIBUTION SYSTEM

#### 3.1 GENERAL

A few RWS schemes have been taken from U.P. Jal Nigam to get their water distribution networks analysed as discussed in previous chapter. These schemes are traditionally designed and there is no suitable and accurate method in the Department to analyse the schemes. While urban water supply schemes receive considerable attention, in terms of efficient analysis and design, there appears to be no such attempts towards optimal design in RWSS, even though the cost involved is in no way a small amount. So, it is essential to analyse the rural water supply distribution network to meet the flow and pressure requirements and to make the scheme optimal as far as feasible.

#### 3.2 NETWORK ANALYSIS PROGRAMME

An efficient computer programme for the analysis of pressure and flow in pipe distribution systems, prepared by University of Kentucky, was available with Dr. K. Subramanya. This programme was suitably modified and implemented on the IIT Kanpur computer system, DEC 1090. This programme gives rapid and accurate results in only few trials. Features of this programme are given in Appendix I. The computer programme is written in FORTRAN IV, G level. It consists of a main



programme and five sparse matrix subroutines to solve the  $p$  linearized simultaneous equations where  $p$  is the number of pipes in the system. Before going to the programme a few terms related to distribution network are being defined.

(i) Node

The end points of pipes are called nodes.

(ii) Junction Node

It is a node where two or more pipes meet or where flow is put in or removed from the system. If a pipe diameter changes occurs at a component such as a valve or a pump, this point is a junction node.

(iii) Fixed Grade Node

It is node in the system where both the pressure and elevation (or hydraulic grade line i.e. piezometric head) are known. This is usually the surface of a storage tank or reservoir or a source or discharge point of specified pressure. Each system must have at least one fixed grade node.

(iv) Primary Loop

It is a closed pipe circuit with no closed pipe circuits contained within it.

If the junction, primary loops, and fixed grade nodes are identified as described here the following holds for all pipe system:



$$p = j + l + t - 1 \quad (3.1)$$

where,  $p$  = number of pipes

$j$  = number of junction

$l$  = number of primary loops

and  $t$  = number of fixed grade nodes.

#### (v) Pipe System Components

##### Pipes:

The length, inside diameter and roughness of each pipe must be input as data. One can use roughness coefficient both for Hazen Williams equation and Darcy-Weisbach equation.

##### Pumps:

A pump can be included in any line of the pipe system. The characteristics of the pump can be input in two ways :

- i) The power the pump puts into the system (in Horse power or kilowatts)
- ii) Minimum three points of operating data (Head-discharge) under normal operating range. This program is designed to work also if the points are outside the normal operating range. A second order curve can be fit to this data to obtain a pump characteristic curve describing the pump operation of the forms

$$E_p = A + BQ + CQ^2 \quad (3.2)$$



where A, B, C, are the characteristic coefficients  
and  $E_p$  is the head corresponding to the discharge Q.

#### Minor Loss:

A number of components in a pipe system (such as valves, junctions, bends, meters etc.) produce a head loss, calling minor loss. It could be input easily

$$H_{LM} = M \frac{V^2}{2g} \quad (3.3)$$

where M = minor loss coefficient

V = line velocity

and g = acceleration due to gravity.

#### Check Valve:

These valves allow flow only in the specified direction. If flow reversal occurs the valve shuts and the line causes no flow.

#### Pressure Regulating Valves (PRV's):

These valves are designed to maintain a specified discharge pressure (PR) which is lower than the pressure upstream from PRV.

### 3.3 FORMULATION FOR THE ANALYSIS

#### 3.3.1 Basic Equations

Equation (3.1) giving the relationship between the



number of pipes, loops, junctions, and fixed grade nodes becomes significant when formulating a proper set of hydraulics equations to describe a pipe system.

In terms of the unknown discharge in each pipe, a number of continuity and energy equations can be written equating the number of pipes in the system.

i) Continuity Equation:

For each junction node the flow into the junction should be equal to the flow out of the junction, written as follows:

$$Q_{in} = Q_{out} \quad (j \text{ equations}) \quad (3.4)$$

ii) Energy Equations:

For each loop the sum of head loss around a loop should be equal to the sum of the energy put into the loop liquid by a pump. So for each loop the energy equation can be written as follows:

$$\sum h_L = \sum E_p \quad (l \text{ equations}) \quad (3.5)$$

where  $h_L$  = head loss in each pipe (including minor loss)

$E_p$  = energy put into the liquid by a pump.

If there are no pumps in the loop then the energy equation will state that the sum of the head loss around a loop equals zero.



If there are  $t$  fixed grade nodes,  $t-1$  energy equations can be written for paths between any two fixed grade nodes as follows:

$$E = \sum h_L - \sum E_p \quad (t-1 \text{ equation}) \quad (3.6)$$

where  $E$  = grade difference between the two fixed grade nodes.

Any path in the pipe system can be chosen between these nodes. However, care must be taken to avoid redundant paths. The best method to avoid this difficulty is to either choose all paths starting at one source (like A-B, A-C, A-D, etc.) or to use the previous end point for a path as the starting point for the next path (like A-B, B-C, C-D, etc.). Either of these methods will result in  $t-1$  equations with no redundant ones. These junction equations (continuity equations) and loop and path equations (energy equations) constitute a set of simultaneous nonlinear equations which can be solved for the discharge in each line.

### 3.3.2 Direct Solution of Linearized Equations

Because of the nonlinear nature of the above equations a direct solution is not possible. A linearization procedure is used to handle the non-linear head loss and pump terms so the system of equations can be cast as a set of  $p$  linear simultaneous equations which can be solved by routine matrix methods. Essentially the technique used to solve the system equations is this.



Based on an assumed flow in each line (a velocity of 4 units in each pipe is used) the non-linear hydraulic equations are linearized and the linearized equations are simultaneously solved for the flowrates. This set of flowrates is used to linearize the equations and a second solution is obtained. The procedure is repeated until the change in flowrates obtained in successive trials is insignificant. Because all flows are computed simultaneously, convergence is assured and occurs very fast compared to other procedures. Usually only 4-6 trials are required even for large systems. In the present study for the large system, 2 trials were required in tree type configuration and 6 trials in looped type configuration.

### 3.4 COMPUTER FLOW DIAGRAM

The simplified computer flow diagram has been given on page 49 .



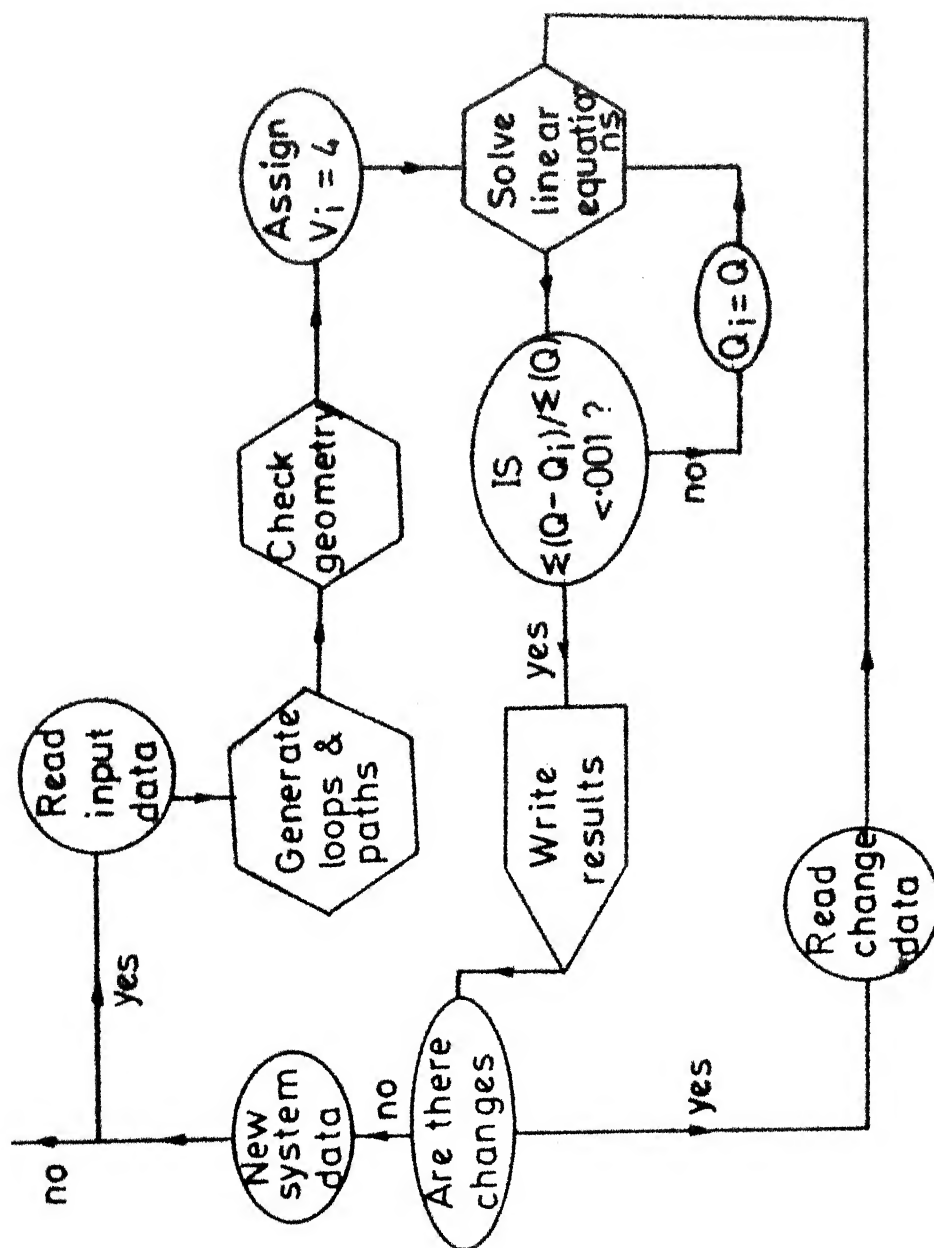


FIG.3-1 SIMPLIFIED COMPUTER FLOW DIAGRAM



### 3.5 BASIC EQUATIONS

The line head loss is given by

$$h_{LP} = K_P Q^n \quad (3.7)$$

where  $K_P$  = pipe line constant

$Q$  = discharge in the pipe line

and  $n$  = exponent

For Hazen Williams equation

$$K_P = \frac{K_1 L}{C^{1.852} D^{4.87}} \quad (3.8)$$

and  $n = 1.852$

where  $L$  = pipe line length

$C$  = Hazen Williams Roughness coefficient

$D$  = internal pipe diameter

and  $K_1$  is a coefficient

= 10.69 for SI units

= 4.73 for English units

For Darcy Weisbach Equation

$$K_P = \frac{K_2 fL}{D^5} \quad (3.9)$$

and  $n = 2.0$

where  $f$  = friction factor

$L$  = pipe line length

$D$  = internal pipe diameter



and  $K_2$  is a coefficient  
 $= 0.08265$  for SI units  
 $= 0.02517$  for English units

Minor losses are given by a loss coefficient,  $M$ , which multiplies the velocity head to give the loss at the component. This is

$$h_{LM} = M \frac{V^2}{2g} \quad (3.10)$$

where  $V$  = the mean line velocity  
and  $g$  = the gravitational constant

In terms of the discharge this is

$$h_{LM} = K_M Q^2 \quad (3.11)$$

$$\text{where } K_M = \frac{K_3 M}{D^4} \quad (3.12)$$

$Q$  = discharge in pipe line  
 $D$  = internal dia of the pipe line  
and  $K_3$  is a coefficient  
 $= 0.08265$  for SI units  
 $= 0.02517$  for English units

The effect of minor loss will be insignificant on the nodal pressure heads so it has not been considered in the present study.

The pump head is expressed in two ways:

- i) When pump power is given, general equation for pump head is

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$$E_P = \frac{P}{\gamma Q} \quad (3.13)$$

where  $P$  = power of the pump in watts

$\gamma$  = specific weight of the liquid =  $\gamma_w S$

$Q$  = discharge in the pipe line

$\gamma_w$  = specific weight of the water

and  $S$  = specific gravity of the liquid

$$\text{So, } E_P = \frac{P}{\gamma_w S Q}$$

Here, for SI units

$$E_P = \frac{1000 P}{9806 S Q}$$

$$\text{or } E_P = \frac{0.10197 P}{S Q} \quad (3.14)$$

and for English units

$$E_P = \frac{550 P}{62.42 S Q}$$

$$\text{or } E_P = \frac{8.814 P}{S Q} \quad (3.15)$$

ii) When operating points for the pump are given, the pump head is

$$E_P = A + BQ + CQ^2 \quad (3.16)$$

where  $A$ ,  $B$ , and  $C$  are the coefficients of a parabolic characteristic curve which defines the pump operation in the vicinity of the operating point.



Since this expression is only valid over a specified range it should not be indiscretely employed in an analysis.

The basic energy equation for a loop or a path between fixed grade nodes is

$$(h_{LP} + h_{LM}) = E + \sum E_P \quad (3.17)$$

where  $E$  is the energy difference between the fixed grade nodes.

This equation can be linearized with the continuity equations to get a set of  $p$  simultaneous linear equations in terms of the flowrates in each pipe using a gradient approximation.

### 3.6 IMPLEMENTATION OF THE PROGRAMME

This original programme is fairly lengthy involving 1,117 FORTRAN cards. The programme was created on the disk of the DEC 1090 system through the cards, and editing was done through the terminals (tty). After compressing the programme file, it occupies 62 blocks storage on disk. The syntax errors were corrected through the terminals. Considerable amount of time was spent on implementing the programme on the DEC system.

#### 3.6.1 Modification due to Computer System

A large number of changes in the programme had to be made to implement it on the DEC system. The major changes are:



- i) '\*2' was removed from the programme and 'REAL\*8' and 'REAL' were substituted by 'DATA'.
- ii) In READ and WRITE statements device unit numbers were removed and only format statement number were given, also the 'WRITE' was changed to 'PRINT'.
- iii) The cards were arranged in the following order:
  - (a) Name of subroutine;
  - (b) Dimension cards;
  - (c) Variable declaration cards, REAL and INTEGER;
  - (d) Common cards;
  - (e) DATA statement cards;
  - (f) Programme card.

### 3.6.2 Additions

Some additions were made to run the programme and to get the desired results for the present study. These are:

- i) Array 'ROWCOL(2)' and 'NAME(5)' were introduced in the dimension statements in the subroutines 'MA18A' and 'MA18B' respectively.
- ii) Subroutine COSTT

This subroutine was added to get the total cost of the distribution network. We can give the rate/m length of the pipe for 20 standard pipe diameters. The cost was calculated nicely to know the cost of the scheme just after each analysis.



By making some changes in the system we will get next new cost of the scheme each time. This was felt to know the cost of the scheme when an round off optimized network was being obtained by the provision of putting changes in the system.

iii) Some FORMAT and PRINT statements were introduced to get the cost output and 'nice prints out' for the U.P. RWS schemes.

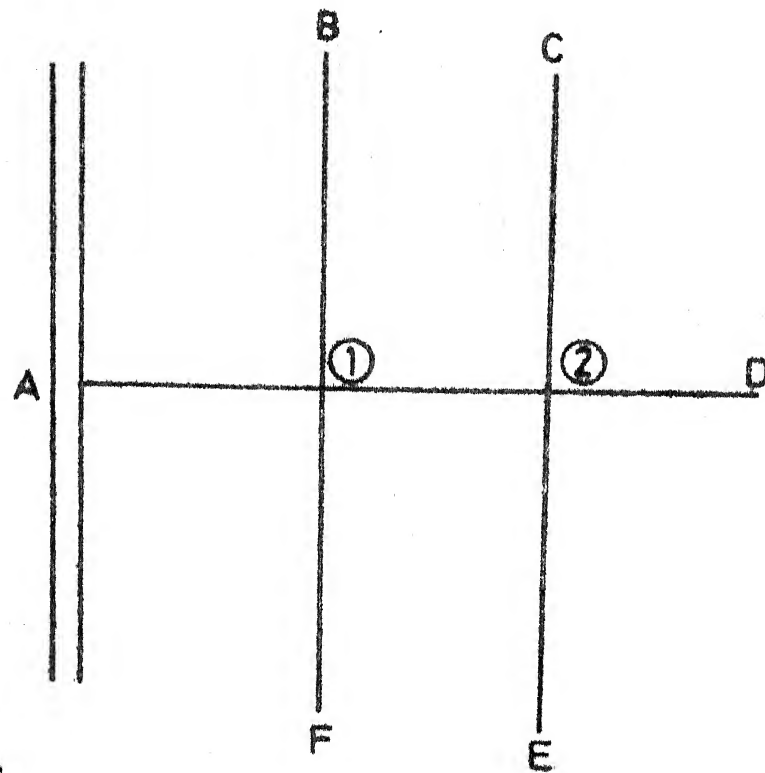
### 3.6.3 Computation Time for a Big Scheme

All the five RWS schemes were analysed on DEC 1090 system. The Malasa Group of villages RWSS is biggest one. The distribution network of this scheme is of loop type. It has 298 pipes. It has been observed that the analysis of loop type network of the same number of pipes takes more CPU time than tree type network, as previous type network takes more number of trials to get the desired accuracy limit. The distribution network of Malasa, RWSS took 7.41 seconds CPU time for its analysis. The initial analysis took 5 trials to get the accuracy of 0.00363 l/s. The analysis after changes takes only 2 trials to get the accuracy of 0.00297 l/s. The execution for this scheme uses CORES of 100P.

### 3.7 TESTS

The programme was tested for four different types of water distribution networks. The details of the problems





Fixed grades at

A = 253.62

B = 107.87

C = 80.90

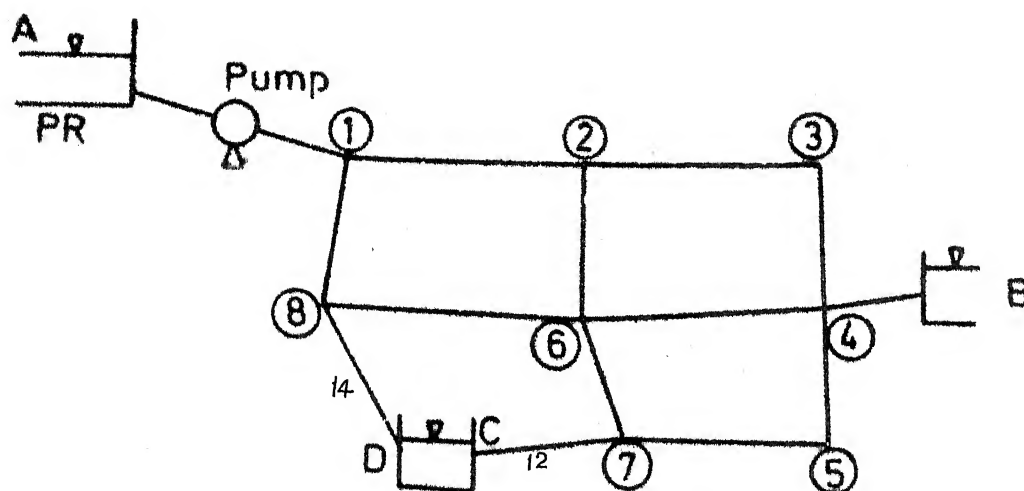
D = 121.81

E = 43.98

F = 100.00

Fig. 3.2 Schematic diagram of tree type distribution system fed by a large pressure main – English Units,  $p=7$ ,  $l=0$ ,  $j=2$ , and  $t=6$





Fixed grades at

A = 30.05

B = 30.48

C = 33.53

D = 33.53

Pump characteristics

E in (m)	Q (l/s)
165.61	200.00
131.85	600.00
17.83	1000.00

Fig.3.3 Schematic diagram of loop type fourteen pipe distribution system -SI Units,  $p=14$ ,  $l=3$ ,  $j=8$  and  $t=4$



tested with computer results are as follows:

### 3.7.1 Test Problem No.1

This seven pipe tree type system (Fig. 3.2) is fed by a pressurized main at a constant pressure of 60 psig. All discharge points are fixed grade nodes, labeled A-F. The liquid used is gasoline having specific gravity of 0.68. Here the Darcy Weishbach equation was used in analysis. DEC 10 has taken only 3 trials against 12 trials (previously tested problem ) and gave better accuracy limit.

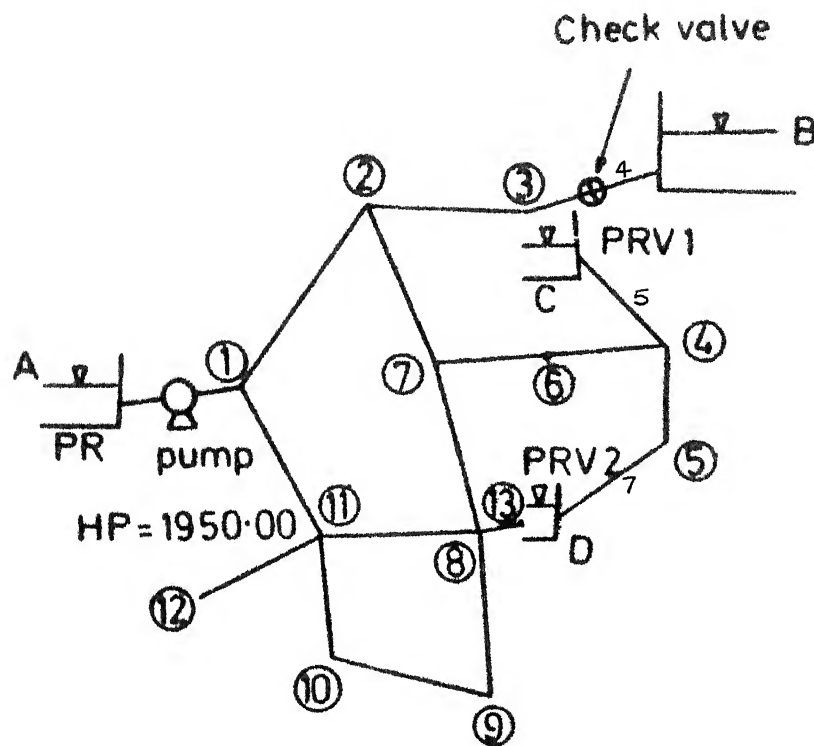
### 3.7.2 Test Problem No.2

This fourteen pipe system has 3 loops and 4 fixed grade nodes, labeled A, B, C and D (Fig. 3.3). Pipe numbered 12 and 14 both are connected to the same storage reservoir. There is a pump in line 1 and has been described by operating data. Here an option for multiplying factor to the changes for demands is used. The pump operates within the allowable range. It has taken only 3 trials against 4 trials (previously tested problem).

### 3.7.3 Test Problem No.3

The system has a pump described by the useful horsepower. An item of note is a check valve in line 4, which allows flow only in the direction towards the storage





Fixed grades at

A = 50.00

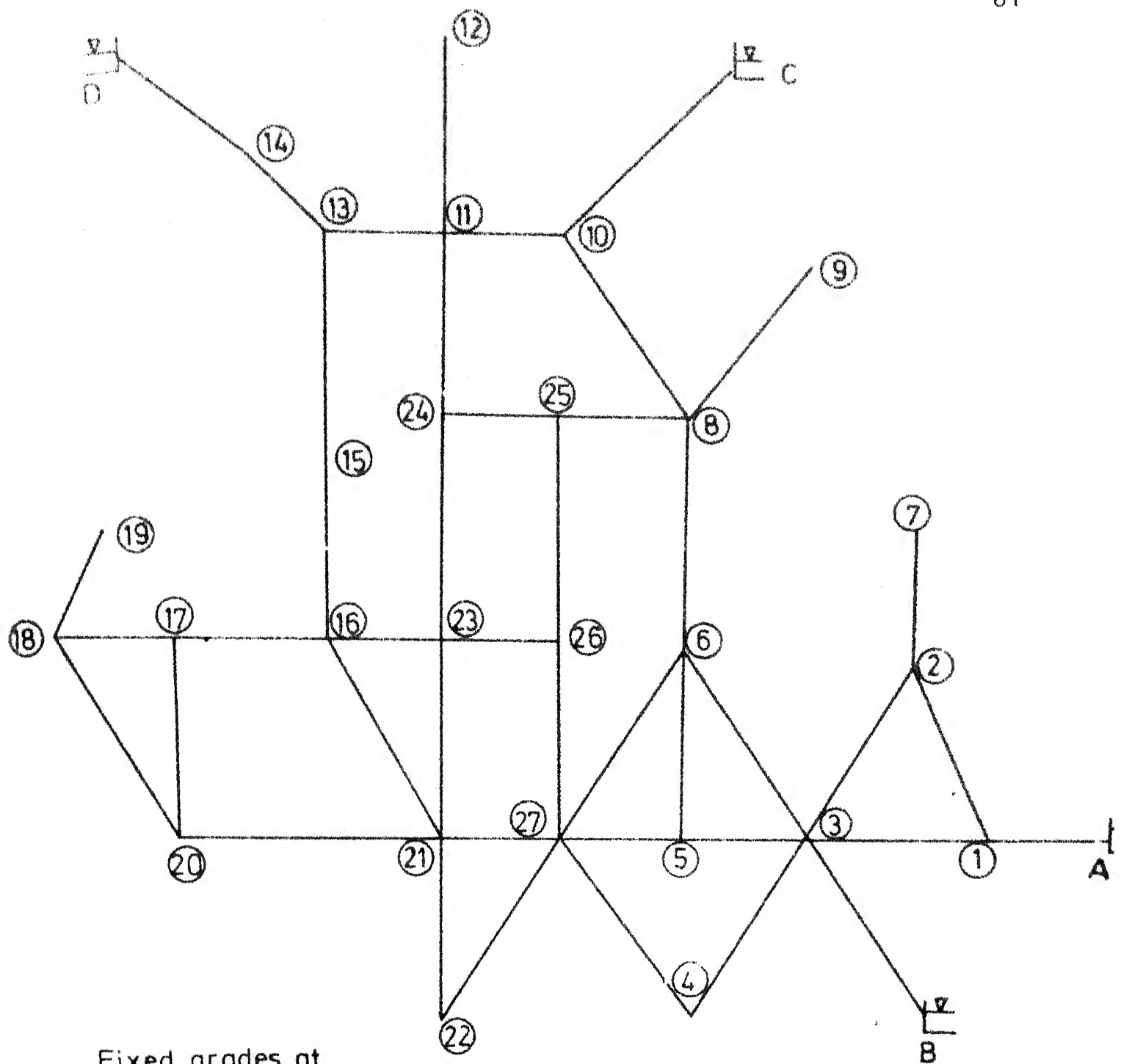
B = 200.00

C = 303.46

D = 278.46

Fig.3 4 Schematic diagram of pipe water distribution system having one check valve, one pump and 2 PRV's -English Units,  $p=18, l=2, j=13, t=4$





Fixed grades at  
 A = 972.00  
 B = 935.00  
 C = 935.00  
 D = 935.00

Fig.3.5 Schematic layout of improved water distribution system for the city of Montpelier, VT. English Units,  $p = 43$ ,  $l = 13$ ,  $j = 27$  and  $t = 4$



is 7 m and 11 nodes give less pressure heads. By making changes as shown in the computer print out the scheme has been made safe giving insignificant increase in the cost of the scheme. It took only 2.87 seconds CPU time for this analysis.

#### 3.8.2 Scheme 2

This scheme was found to be over-designed. The terminal pressure were coming more than specified, 7m. It took 1.00 second CPU time. A little cost was reduced after changes to make the network nearly optimal.

#### 3.8.3 Scheme 3

The analysis of this scheme gave more negative pressure at some nodes . One complete branch of the network was under-designed. This 138 pipes tree type network has taken 2.59 seconds CPU time for its analysis. The cost of the scheme has been increased by 1.73 percent to be safe.

#### 3.8.4 Scheme 4

It is the biggest scheme. It comprises 298 pipes and 56 loops. It took 7.73 seconds CPU time. It was more under-designed and closely optimized scheme needed the scheme cost increased by 2.85 percent. Due to loop type it has taken 5 trials for initial analysis.



### 3.8.5 Scheme 5

It was too much over-designed and after making changes to get nearly optimal network, the saving turned out to be 11.80 percent. It took 2.76 seconds CPU time for analysis.



## 4 DESIGN OF TREE TYPE DISTRIBUTION SYSTEM

### 4.1 GENERAL

The essential part of all water supply schemes is the water distribution system. It comprises a considerable part of the total expenditure incurred on water supply schemes. It is apparent from the comparative study of a few U.P. rural water supply schemes, randomly selected, that 65 percent or more of the total cost of the scheme goes to the distribution system. So, optimization of the distribution network is highly desirable.

### 4.2 PROBLEM

The distribution systems are of mainly two types: (i) loop type and (ii) tree type. As alternate routes are available for transporting water to the demand nodes in looped systems, they are more reliable, though somewhat costlier, than the other type. Reliability is preferred to economy and also the street pattern is grid like for large communities and therefore, loop type networks are employed for urban water supply schemes. For rural water supply schemes, however cost rather than reliability is the design criterion, and also the pattern of villages is of zig-zag nature, so, tree type system is used for them.



Previously all water supply schemes were concerned with Urban communities only. But nowadays worldwide agitation is being made to provide safe drinking water to all rural communities by 1990. In view of vast financial outlay in these projects we have to select optimized distribution network in all schemes.

We can have two ways of optimizing the network systems by (i) global optimal design and (ii) nearly optimized design. Optimal design is really a hard task and can be only possible by an efficient computer programme using the theory of Linear Programming. The other type of design which is to be nearly optimum. This is specifically studied here.

#### 4.3 PAST WORKS

A review of past work in this field is made here before going to the design problem for tree type network systems. Bhav<sup>2</sup> used the method of critical path approach for the design of dead-end system. He showed that an optimal solution could be obtained directly without trial and error by system reduction procedure and by adopting an economic friction slope. The friction slope is assumed. So, one can not be certain that the optimal solution is achieved, though it may not be far from it. Deb<sup>9</sup> presented a



method to get optimal solution of the branched pipe network system. He assumed a boundary condition that the total head-loss of the system through any branch is same. Khanna and Swamee<sup>11</sup> suggested an optimal design problem which reaches to a non-linear constraints involving an unknown procedure for solution and the work is based on simplifying the problem through approximations. Bhave<sup>1</sup> has a non-computer approach having steady flows. The approach is based on the critical path method, the method is simple, straight forward and gives a fairly economical distribution method without any trial and error procedure. Deb<sup>8</sup> suggested that for optimal design of the water distribution system, the initial costs of the pipes, pumps and elevated service reservoir, and maintenance and electricity costs should be considered. According to him the total cost of a water distribution system is dependant on the inlet hydraulic gradient and the pressure surface geometry. With the increase of inlet hydraulic head and the pressure surface parameter (the ratio of the actual slope of the pressure profile at the farthest point from the inflow to the straight line variation slope) value, the total cost decreases initially, reaches a minimum value, and then increases. Optimum values of inlet hydraulic head,  $H_I$  and pressure surface parameter,  $S_R$  could be obtained. His study also found that the position of the elevated service



reservoir within the network is an important factor in cost optimization. If the position of the service reservoir is removed from the corner of a rectangular pipe network, towards the centre along the diagonal, the distance between the inlet point and the farthest point in the network is reduced, and for a specified head, the hydraulic gradient is increased. As a result, pipe cost is expected to be reduced. It was found that the total cost of the system is found to be 1.33 times more when the reservoir position is at one corner of the network than when it is at the centre.

#### 4.4 CURRENT PRACTICE

From the study of five RWS schemes of the State, U.P., we see that the distribution networks were designed traditionally. In design, the tables based on Hazen Williams Formula were used. As the tables and graphs are used in design, there is a more chance to commit error in selecting a correct pipe diameter for the flow and friction slope conditions. There is no provision to check the error and it will be commulative giving more and more errors. For example, in the design of distribution network of Urwa, Part II, group of villages RWWS, a mistake has been committed in the beginning of a branch having starting node number 78. Due to this mistake the whole branch starting from node number 78 is unsafe from the



terminal pressure head point of view. There is no suitable method to analyse the distribution network after designing it. In other schemes under study, some are over-designed and some under-designed. So, there is an essential need of a proper analysis for the flow and pressure conditions of the network and a proper design procedure.

#### 4.5 SUGGESTED IMPROVEMENT

By the analysis of distribution network we get the terminal pressure condition. Observing the pressure condition we have to change the pipe diameter upstream of the corresponding terminal node, to get the network improved. So, no doubt, the analysis designs the network system in other way.

The computer programme used for the analysis of the scheme here was used to design the network and make it as for as possible optimal. The final costs of the schemes were calculated after the suggested design. The computer programme can be efficiently used to make the system near about optimal which is previously arbitrarily designed. This has been done in Urwa, Part II, group of villages, RWSS, Allahabad.

#### 4.6 A SUGGESTED LP METHOD

##### 4.6.1 General

A method based on the critical path concept can be



developed for selection of the optimal sets of pipe sizes for optimization of branching networks by LP. The number of pipes of consecutive sizes in an optimal set depends upon the quality of optimality needed. Practically two pipe sizes are satisfactory for a system. For global optimality one can select four, or even more pipe sizes. Before going to the critical path concept and behaviour of LP, we will be clear about some definitions.

#### Path

In branching system there is only one route to each demand node (terminal node) from the source. This route is called path.

#### Slope of Path

This is maximum available average friction slope.

This is expressed as

$$S = \frac{H_0 - H^{\min}}{L_p} \quad (4.1)$$

where  $S$  = slope of the path

$H_0$  = HGL at the source

$H$  = minimum required HGL at the demand node

and  $L_p$  = length of the path

#### Critical Slope

The minimum of all the path-slopes is termed as critical path.



$$S_c = \text{Minimum} \frac{H_o - H^{\min}}{L_p}, \text{ giving } S_c \leq S \quad (4.2)$$

#### Critical Path and Critical Node

The path having the maximum available average friction slope equal to the critical slope is termed as critical path and the node at the end of the critical path is termed as critical node.

#### Link

It consists of one or more pipes connected in series and has a constant flow and no branches.

#### 4.6.2 LP Model

##### Decision Variable

As the resistance of a pipe and its cost are linear functions of its length, the different pipe lengths constituting a link are taken as the decision variables.

##### Objective Function

The objective function is to minimize the sum of the capital costs of the various links.

##### Constraints

(i) For all links the sum of the lengths of the pipes selected must equal the length of the link.



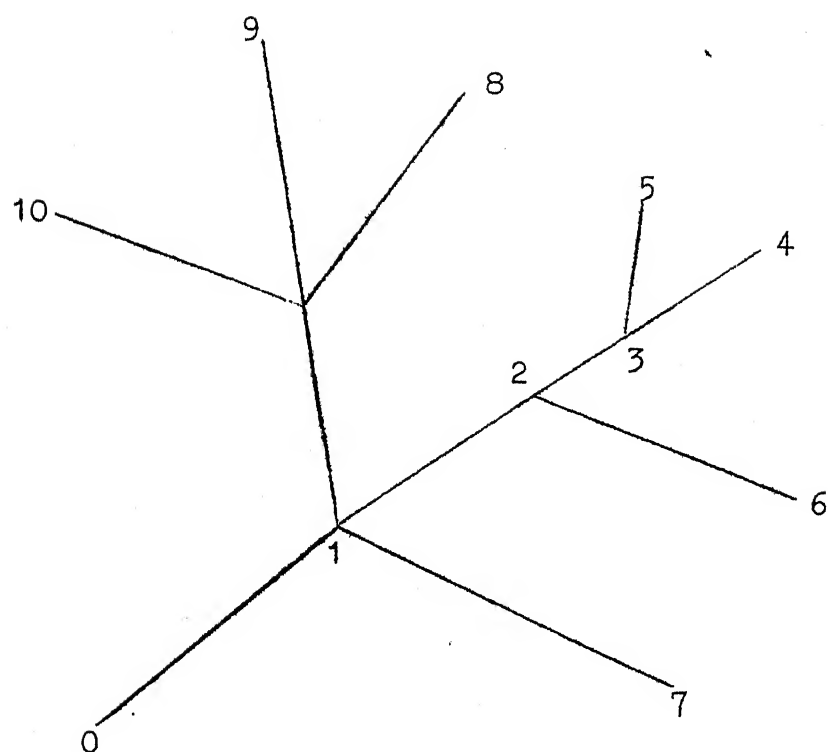


FIG. 4.1: BRANCHING DISTRIBUTION SYSTEM



(ii) At each demand node the available HGL value must be equal to or greater than the minimum required HGL value at the node.

(iii) All pipe-lengths must be non-negative.

By experience, a designer, who carries out the network optimization, selects generally an optimal set of two or three pipe sizes, but this set may not constitute the optimal set. So, critical path concept is suggested here to select optimal set of pipe sizes.

#### 4.6.3 Critical Path Concept

Here, the critical path for the entire system is located. There will be so many paths as demand nodes. The critical one is chosen in the system. For a node  $i$  on the critical path, the proposed HGL value  $H_i$  will be given by

$$H_i = H_o - S_c L_{Pi} \quad (4.3)$$

From equation (4.1)

$$H_i^{\min} = H_o - S_i L_{Pi} \quad (4.4)$$

From equations (4.3) and (4.4)

$$H_i \geq H_i^{\min}$$

which satisfy the node HGL constraints of the optimization problem, for all the nodes on critical path.



Thus , when all the links on the critical path are provided with critical slope, the node HGL constraints for all the nodes on the critical path are satisfied, and the flow takes place along the other paths.

After deciding the critical path and estimating the HGL values for the nodes lying on it, the critical path is removed from the distribution system. This leaves behind distribution sub-systems of the first order. These distribution sub-systems emerge from the nodes of known HGL values and thus are treated as independent distribution subsystems for which critical subpaths are located and the HGL values for nodes lying on them are estimated. The procedure of obtaining the distribution subsystems of first and higher orders, locating the critical subpaths and estimating the HGL values for nodes lying on them is continued until the proposed friction slopes for all links of the entire system can be estimated.

Practically the commercial pipe sizes which have friction slopes equal to the immediate lower and higher values constitute the optimal set for each link.

#### 4.6.4 Formulation

After getting the critical path, the diameter of all the links lying on that path will be calculated by



$$D = \left( \frac{10.69 Q^{1.852}}{C^{1.852} S} \right)^{1/4.87}$$

where  $Q$  = pipe discharge,  $m^3/sec.$

$C$  = Hazen Williams constant

$S$  = friction path slope

and  $D$  = internal diameter of the pipe in m

The set of two commercial pipe sizes having immediate lower and higher values than the calculated size will be optimal set.

#### 4.6.5 Computer Programme

On the basis of this LP model based on critical path concept a computer programme in FORTRAN IV language was prepared. This consists of one main program and one subroutine dia. This has been created on the disk of IIT/K DEC 1090 system, and has been found to work satisfactorily. The programme is however in the initial stage of development and needs full testing. The details are available with Dr. K. Subramanya.



## 5 CONCLUSIONS AND RECOMMENDATIONS

Five rural water supply schemes under U.P. Jal Nigam were selected for the present study. Three tree type schemes are in district Allahabad named Girdkot, Ketehra and Urwa and two loop cum tree type schemes are in district Kanpur named Malasa and Rajpur. The topography of the areas where these schemes fall are more or less flat. Overall comparative study of these schemes has been done. The need for an efficient design and analysis of water distribution networks has been brought out.

These schemes were analysed by an efficient modified water distribution network system analysis computer programme. By the analysis of these schemes, it was found that some schemes were under-designed and some over-designed. These deficiencies of the existing schemes were removed with the help of the programme. By making suitable changes in the pipe line data of the schemes, in few trials we have got nearby optimized networks and final cost was calculated for each scheme. On the basis of the comparative study of the schemes, it has been found that the per capita expenditure for a typical scheme, is Rs. 105/- for the year 2001.

A computer method for the optimum design of tree type water distribution network has been indicated. This



method is based on critical path approach.

It has been found that:

- i) There is scope for considerable improvement in the schemes to effect economy.
- ii) Computer analysis and design is a necessity and the traditional methods of design ~~have~~ to give way to this.
- iii) The analysis programme used in the present study is very fast, versatile, reliable and could be used by Jal Nigam and other organisations connected with water supply for obtaining status of their design/systems.
- iv) There is a need for a computer based optimal water supply design programme.



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## APPENDIX I - FEATURES OF NETWORK ANALYSIS PROGRAMME

Features of the programme can be listed as follows:

1. Any type of pipe system configuration can be handled.
2. The system can contain any number of storage tanks, pumps, valves, meters, fittings etc.
3. Pumps can be described by useful power or by inputting head-flow data from operating curves. Out of range pump operation is incorporated into the programme.
4. The system can have pressure regulating valves which isolate entire low pressure regions or various single pressure regulations described throughout the system.
5. Check valves, which allow flow in only one direction, can be included.
6. Flow units of CFS, GPM, MGD or SI can be used.
7. Data preparation is simple even for large systems.
8. Complete output is provided including pressures, elevations and grade lines at all junctions, head losses in lines and at all valves, pump heads, flowrates and velocities.
9. The procedure is relatively fast. Typical computer times for execution on DEC 1090 system at IIT Kanpur are as follows:

For loop systems: 2-3 seconds for 100 pipes network

6-7 seconds for 300 pipes network



For tree type systems : 1-2 seconds for 100 pipes network.

10. It has the characteristics to converge to a solution for all situations.
11. No assumptions (such as initial flowrates or pressures) are required.
12. Sparse matrix routines are used which minimize storage requirements and increase computer execution times.
13. A pipe system of  $p$  pipes requires approximately  $50 p$  dimensioned storage (word) spaces.

The basis of the programme is a direct solution of the basic pipe system hydraulic equations using a linearization scheme<sup>(A-1)</sup> and sparse matrix methods to handle the non-linearization methods to solve the equations. It utilizes linear network theory.

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Ref:(A-1): Wood, Don J., and Charles, Carl, O.A., 'Hydraulic Network Analysis Using Linear Theory,' Journal of Hydraulics Division, ASCE, Vol.98, No.HY7, Proc. Paper 9031, July 1972, pp. 1157-1170.



## APPENDIX II - RESULTS OF ANALYSIS

Typical output results relating to the analysis of all five U.P. Jal Nigam schemes (A through E) are appended.







INJECTION NO.	DEMAND (GPI/SEC)	GRADE LINE (FEET FRS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	44.407	29.440	14.97
2	0.00000	41.500	29.250	12.03
3	0.00000	41.404	29.370	19.03
4	0.00000	40.340	30.430	9.01
5	0.00000	39.367	31.420	7.95
6	0.00000	38.013	31.420	7.49
7	0.00000	37.724	30.440	11.19
8	0.00000	41.633	30.440	11.19
9	0.00000	41.710	30.380	10.26
10	0.00000	40.665	30.400	8.59
11	0.3583	39.131	30.270	8.74
12	0.00000	40.073	30.330	8.21
13	0.3889	38.553	30.220	8.00
14	0.32000	38.000	30.220	8.00
15	0.00000	38.000	30.220	8.00
16	0.00000	38.000	30.220	8.00
17	0.00000	38.000	30.220	8.00
18	0.00000	38.000	30.220	8.00
19	0.00000	38.000	30.220	8.00
20	0.00000	38.000	30.220	8.00
21	0.00000	38.000	30.220	8.00
22	0.00000	38.000	30.220	8.00
23	0.00000	38.000	30.220	8.00
24	0.00000	38.000	30.220	8.00
25	0.00000	38.000	30.220	8.00
26	0.00000	38.000	30.220	8.00
27	0.00000	38.000	30.220	8.00
28	0.00000	38.000	30.220	8.00
29	0.00000	38.000	30.220	8.00
30	0.00000	38.000	30.220	8.00
31	0.00000	38.000	30.220	8.00
32	0.00000	38.000	30.220	8.00
33	0.00000	38.000	30.220	8.00
34	0.00000	38.000	30.220	8.00
35	0.00000	38.000	30.220	8.00
36	0.00000	38.000	30.220	8.00
37	0.00000	38.000	30.220	8.00
38	0.00000	38.000	30.220	8.00
39	0.00000	38.000	30.220	8.00
40	0.00000	38.000	30.220	8.00
41	0.00000	38.000	30.220	8.00
42	0.00000	38.000	30.220	8.00
43	0.00000	38.000	30.220	8.00
44	0.00000	38.000	30.220	8.00
45	0.00000	38.000	30.220	8.00
46	0.00000	38.000	30.220	8.00
47	0.00000	38.000	30.220	8.00
48	0.00000	38.000	30.220	8.00
49	0.00000	38.000	30.220	8.00
50	0.00000	38.000	30.220	8.00
51	0.00000	38.000	30.220	8.00
52	0.00000	38.000	30.220	8.00
53	0.00000	38.000	30.220	8.00
54	0.00000	38.000	30.220	8.00
55	0.00000	38.000	30.220	8.00
56	0.00000	38.000	30.220	8.00
57	0.00000	38.000	30.220	8.00
58	0.00000	38.000	30.220	8.00
59	0.00000	38.000	30.220	8.00
60	0.00000	38.000	30.220	8.00
61	0.00000	38.000	30.220	8.00
62	0.00000	38.000	30.220	8.00
63	0.00000	38.000	30.220	8.00
64	0.00000	38.000	30.220	8.00
65	0.00000	38.000	30.220	8.00
66	0.00000	38.000	30.220	8.00
67	0.00000	38.000	30.220	8.00
68	0.00000	38.000	30.220	8.00
69	0.00000	38.000	30.220	8.00
70	0.00000	38.000	30.220	8.00
71	0.00000	38.000	30.220	8.00
72	0.00000	38.000	30.220	8.00
73	0.00000	38.000	30.220	8.00







17493-00  
17082-90  
5561-00  
12871-00

TOTAL COST =

342437.70



THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	CODE NUMBERS	LENGTH (METERS)	DIAMETER (CM)	ROUGHNESS
28	5	250.0	140.0	140.0
29	31	312.0	140.0	140.0
31	33	162.0	140.0	140.0
32	34	300.0	140.0	140.0
33	35	20.0	140.0	140.0
34	36	150.0	140.0	140.0
35	37	20.0	140.0	140.0
36	38	20.0	140.0	140.0
37	39	20.0	140.0	140.0
38	40	20.0	140.0	140.0
39	41	20.0	140.0	140.0
40	42	20.0	140.0	140.0
41	43	20.0	140.0	140.0
42	44	20.0	140.0	140.0
43	45	20.0	140.0	140.0
44	46	20.0	140.0	140.0
45	47	20.0	140.0	140.0
46	48	20.0	140.0	140.0
47	49	20.0	140.0	140.0
48	50	20.0	140.0	140.0
49	51	20.0	140.0	140.0
50	52	20.0	140.0	140.0
51	53	20.0	140.0	140.0
52	54	20.0	140.0	140.0
53	55	20.0	140.0	140.0
54	56	20.0	140.0	140.0
55	57	20.0	140.0	140.0
56	58	20.0	140.0	140.0
57	59	20.0	140.0	140.0
58	60	20.0	140.0	140.0
59	61	20.0	140.0	140.0
60	62	20.0	140.0	140.0
61	63	20.0	140.0	140.0
62	64	20.0	140.0	140.0
63	65	20.0	140.0	140.0
64	66	20.0	140.0	140.0
65	67	20.0	140.0	140.0
66	68	20.0	140.0	140.0
67	69	20.0	140.0	140.0
68	70	20.0	140.0	140.0
69	71	20.0	140.0	140.0
70	72	20.0	140.0	140.0
71	73	20.0	140.0	140.0
72	74	20.0	140.0	140.0
73	75	20.0	140.0	140.0
74	76	20.0	140.0	140.0
75	77	20.0	140.0	140.0
76	78	20.0	140.0	140.0
77	79	20.0	140.0	140.0
78	80	20.0	140.0	140.0
79	81	20.0	140.0	140.0
80	82	20.0	140.0	140.0
81	83	20.0	140.0	140.0
82	84	20.0	140.0	140.0
83	85	20.0	140.0	140.0
84	86	20.0	140.0	140.0
85	87	20.0	140.0	140.0
86	88	20.0	140.0	140.0
87	89	20.0	140.0	140.0
88	90	20.0	140.0	140.0
89	91	20.0	140.0	140.0
90	92	20.0	140.0	140.0
91	93	20.0	140.0	140.0
92	94	20.0	140.0	140.0
93	95	20.0	140.0	140.0
94	96	20.0	140.0	140.0
95	97	20.0	140.0	140.0
96	98	20.0	140.0	140.0
97	99	20.0	140.0	140.0
98	100	20.0	140.0	140.0
99	101	20.0	140.0	140.0
100	102	20.0	140.0	140.0
101	103	20.0	140.0	140.0
102	104	20.0	140.0	140.0
103	105	20.0	140.0	140.0
104	106	20.0	140.0	140.0
105	107	20.0	140.0	140.0
106	108	20.0	140.0	140.0
107	109	20.0	140.0	140.0
108	110	20.0	140.0	140.0
109	111	20.0	140.0	140.0
110	112	20.0	140.0	140.0
111	113	20.0	140.0	140.0
112	114	20.0	140.0	140.0
113	115	20.0	140.0	140.0
114	116	20.0	140.0	140.0
115	117	20.0	140.0	140.0
116	118	20.0	140.0	140.0
117	119	20.0	140.0	140.0
118	120	20.0	140.0	140.0
119	121	20.0	140.0	140.0
120	122	20.0	140.0	140.0
121	123	20.0	140.0	140.0
122	124	20.0	140.0	140.0
123	125	20.0	140.0	140.0
124	126	20.0	140.0	140.0
125	127	20.0	140.0	140.0
126	128	20.0	140.0	140.0
127	129	20.0	140.0	140.0
128	130	20.0	140.0	140.0
129	131	20.0	140.0	140.0
130	132	20.0	140.0	140.0
131	133	20.0	140.0	140.0
132	134	20.0	140.0	140.0
133	135	20.0	140.0	140.0
134	136	20.0	140.0	140.0
135	137	20.0	140.0	140.0
136	138	20.0	140.0	140.0
137	139	20.0	140.0	140.0
138	140	20.0	140.0	140.0
139	141	20.0	140.0	140.0
140	142	20.0	140.0	140.0
141	143	20.0	140.0	140.0
142	144	20.0	140.0	140.0
143	145	20.0	140.0	140.0
144	146	20.0	140.0	140.0
145	147	20.0	140.0	140.0
146	148	20.0	140.0	140.0
147	149	20.0	140.0	140.0
148	150	20.0	140.0	140.0
149	151	20.0	140.0	140.0
150	152	20.0	140.0	140.0
151	153	20.0	140.0	140.0
152	154	20.0	140.0	140.0
153	155	20.0	140.0	140.0
154	156	20.0	140.0	140.0
155	157	20.0	140.0	140.0
156	158	20.0	140.0	140.0
157	159	20.0	140.0	140.0
158	160	20.0	140.0	140.0
159	161	20.0	140.0	140.0
160	162	20.0	140.0	140.0
161	163	20.0	140.0	140.0
162	164	20.0	140.0	140.0
163	165	20.0	140.0	140.0
164	166	20.0	140.0	140.0
165	167	20.0	140.0	140.0
166	168	20.0	140.0	140.0
167	169	20.0	140.0	140.0
168	170	20.0	140.0	140.0
169	171	20.0	140.0	140.0
170	172	20.0	140.0	140.0
171	173	20.0	140.0	140.0
172	174	20.0	140.0	140.0
173	175	20.0	140.0	140.0
174	176	20.0	140.0	140.0
175	177	20.0	140.0	140.0
176	178	20.0	140.0	140.0
177	179	20.0	140.0	140.0
178	180	20.0	140.0	140.0
179	181	20.0	140.0	140.0
180	182	20.0	140.0	140.0
181	183	20.0	140.0	140.0
182	184	20.0	140.0	140.0
183	185	20.0	140.0	140.0
184	186	20.0	140.0	140.0
185	187	20.0	140.0	140.0
186	188	20.0	140.0	140.0
187	189	20.0	140.0	140.0
188	190	20.0	140.0	140.0
189	191	20.0	140.0	140.0
190	192	20.0	140.0	140.0
191	193	20.0	140.0	140.0
192	194	20.0	140.0	140.0
193	195	20.0	140.0	140.0
194	196	20.0	140.0	140.0
195	197	20.0	140.0	140.0
196	198	20.0	140.0	140.0
197	199	20.0	140.0	140.0
198	200	20.0	140.0	140.0
199	201	20.0	140.0	140.0
200	202	20.0	140.0	140.0
201	203	20.0	140.0	140.0
202	204	20.0	140.0	140.0
203	205	20.0	140.0	140.0
204	206	20.0	140.0	140.0
205	207	20.0	140.0	140.0
206	208	20.0	140.0	140.0
207	209	20.0	140.0	140.0
208	210	20.0	140.0	140.0
209	211	20.0	140.0	140.0
210	212	20.0	140.0	140.0
211	213	20.0	140.0	140.0
212	214	20.0	140.0	140.0
213	215	20.0	140.0	140.0
214	216	20.0	140.0	140.0
215	217	20.0	140.0	140.0
216	218	20.0	140.0	140.0
217	219	20.0	140.0	140.0
218	220	20.0	140.0	140.0
219	221	20.0	140.0	140.0
220	222	20.0	140.0	140.0
221	223	20.0	140.0	140.0
222	224	20.0	140.0	140.0
223	225	20.0	140.0	140.0
224	226	20.0	140.0	140.0
225	227	20.0	140.0	140.0
226	228	20.0	140.0	140.0
227	229	20.0	140.0	140.0
228	230	20.0	140.0	140.0
229	231	20.0	140.0	140.0
230	232	20.0	140.0	140.0
231	233	20.0	140.0	140.0
232	234	20.0	140.0	140.0
233	235	20.0	140.0	140.0
234	236	20.0	140.0	140.0
235	237	20.0	140.0	140.0
236	238	20.0	140.0	140.0
237	239	20.0	140.0	140.0
238	240	20.0	140.0	140.0
239	241	20.0	140.0	140.0
240	242	20.0	140.0	140.0
241	243	20.0	140.0	140.0
242	244	20.0	140.0	140.0
243	245	20.0	140.0	140.0
244	246	20.0	140.0	140.0
245	247	20.0	140.0	140.0
246	248	20.0	140.0	140.0
247	249	20.0	140.0	140.0
248	250	20.0	140.0	140.0
249	251	20.0	140.0	140.0
250	252	20.0	140.0	140.0
251	253	20.0	140.0	140.0
252	254	20.0	140.0	140.0
253	255	20.0	140.0	140.0
254	256	20.0	140.0	140.0
255	257	20.0	140.0	140.0
256	258	20.0	140.0	140.0
257	259	20.0	140.0	140.0
258	260	20.0	140.0	140.0
259	261	20.0	140.0	140.0
260	262	20.0	140.0	140.0
261	263	20.0	140.0	140.0
262	264	20.0	140.0	140.0
263	265	20.0	140.0	140.0
264	266	20.0	140.0	140.0
265	267	20.0	140.0	140.0
266	268	20.0	140.0	140.0
267	269	20.0	140.0	140.0
268	270	20.0	140.0	140.0
269	271	20.0	140.0	140.0
270	272	20.0	140.0	140.0
271	273	20.0	140.0	140.0
272	274	20.0	140.0	140.0
273	275	20.0	140.0	140.0
274	276	20.0	140.0	140.0
275	277	20.0	140.0	140.0
276	278	20.0	140.0	140.0



[illegible][illegible][illegible][illegible][illegible][illegible][illegible][illegible][illegible]

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

[illegible]







105	0.00000	38.489	30.890	7.60
106	0.05833	38.385	30.890	7.50
107	0.00000	38.077	30.700	7.38
108	0.17500	37.246	30.500	7.25
109	0.38889	37.725	30.600	7.33
110	0.05056	37.033	30.700	7.30
111	0.01944	37.986	30.690	7.30
112	0.00000	43.766	29.250	14.52

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CMS)	ROUGHNESS	RATE (/M(RS)	TOTAL LENGTH (METERS)	COST (RS)
25.00	130.0	55.00	0.0	0.00
20.00	130.0	40.00	1913.0	76400.00
15.00	130.0	30.00	1067.0	32010.00
12.50	130.0	25.00	1131.0	28275.00
10.00	140.0	17.80	5241.0	93289.80
8.00	140.0	12.70	3372.0	42818.40
5.00	140.0	7.00	3189.0	22323.00
3.25	140.0	4.70	1172.0	5504.40
	140.0	4.15	1430.0	5921.00
	140.0	4.10	2195.0	9009.25
			3377.0	13845.70
		TOTAL COST=		312355.15



THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CM'S)	THICKNESS
89	107	105.0	3.2	140.0
71	73	300.0	4.0	140.0
662	71	270.0	3.2	140.0
88	83	290.0	3.2	140.0
103	88	78.0	4.0	140.0
20	66	100.0	3.2	140.0
21	66	60.0	3.2	140.0
18	65	90.0	3.2	140.0
25	65	90.0	3.2	140.0
12	62	255.0	5.0	140.0

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

[illegible]



[illegible][illegible][illegible][illegible][illegible][illegible][illegible]

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 125 126 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 157 158 159 160 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191 192 193 194 195 196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246 247 248 249 250 251 252 253 254 255 256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288 289 290 291 292 293 294 295 296 297 298 299 300 301 302 303 304 305 306 307 308 309 310 311 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004 1005 1006 1007 1008 1009 1010 1011 1012 1013 1014 1015 1016 1017 1018 1019 1020 1021 1022 1023 1024 1025 1026 1027 1028 1029 1030 1031 1032 1033 1034 1035 1036 1037 1038 1039 104

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THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

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[illegible]



SCHEME 2

KETEHRRA, ZONE A, GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, ALLAHABAD

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SCHEME HAS 55 PIPES, 55 JUNCTIONS, 0 LOOP AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED APPR 2 RESULTS

PIPE NO.	PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1	1	10.00	15.00	130.00	20.228	0.097	1.145	9.750	117.20
2	2	1	30.00	15.00	130.00	15.117	0.155	0.812	5.158	
3	3	1	35.00	15.00	140.00	4.644	3.537	0.591	4.802	
4	4	1	30.00	15.00	140.00	1.513	2.718	0.459	4.090	
5	5	1	30.00	15.00	140.00	0.284	2.046	0.283	2.392	
6	6	1	30.00	15.00	140.00	0.946	0.126	0.352	1.621	
7	7	1	30.00	15.00	140.00	0.378	2.645	0.170	1.000	
8	8	1	30.00	15.00	140.00	2.266	1.500	0.529	2.578	
9	9	1	30.00	15.00	140.00	2.995	1.467	0.549	2.516	
10	10	1	30.00	15.00	140.00	1.284	1.534	0.378	1.700	
11	11	1	30.00	15.00	140.00	1.049	1.200	0.347	1.460	
12	12	1	30.00	15.00	140.00	5.777	1.200	0.347	1.460	
13	13	1	30.00	15.00	140.00	3.177	1.200	0.347	1.460	
14	14	1	30.00	15.00	140.00	4.027	1.200	0.347	1.460	
15	15	1	30.00	15.00	140.00	1.892	1.200	0.347	1.460	
16	16	1	30.00	15.00	140.00	0.669	1.200	0.347	1.460	
17	17	1	30.00	15.00	140.00	2.821	1.200	0.347	1.460	
18	18	1	30.00	15.00	140.00	0.930	1.200	0.347	1.460	
19	19	1	30.00	15.00	140.00	1.932	1.200	0.347	1.460	
20	20	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
21	21	1	30.00	15.00	140.00	2.884	1.200	0.347	1.460	
22	22	1	30.00	15.00	140.00	1.388	1.200	0.347	1.460	
23	23	1	30.00	15.00	140.00	0.416	1.200	0.347	1.460	
24	24	1	30.00	15.00	140.00	0.265	1.200	0.347	1.460	
25	25	1	30.00	15.00	140.00	0.887	1.200	0.347	1.460	
26	26	1	30.00	15.00	140.00	0.757	1.200	0.347	1.460	
27	27	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
28	28	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
29	29	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
30	30	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
31	31	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
32	32	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
33	33	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
34	34	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
35	35	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
36	36	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
37	37	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
38	38	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
39	39	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
40	40	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
41	41	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
42	42	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
43	43	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
44	44	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
45	45	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
46	46	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
47	47	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
48	48	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
49	49	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
50	50	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
51	51	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
52	52	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
53	53	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
54	54	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	
55	55	1	30.00	15.00	140.00	0.284	1.200	0.347	1.460	

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	117.103	100.000	17.07
2	0.00000	116.818	99.000	17.33
3	0.28375	115.267	99.310	15.06
4	0.00000	111.736	99.790	11.96
5	0.44583	109.399	99.530	11.87
6	1.90112	111.249	99.910	11.34







THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CM'S)	ROUGHNESS
6	5	300.0	3.0	140.0
55	3	400.0	2.5	140.0
31	320	300.0	3.0	140.0
28	32	200.0	2.5	140.0
30	32	210.0	2.5	140.0

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1-2	10.00	15.00	0.00	20.33	0.00	1.14	758	117.20
1	2-3	30.00	15.00	0.00	14.11	1.55	1.81	158	117.20
1	3-4	35.70	10.64	0.00	4.55	1.68	0.97	80	117.20
1	4-5	35.70	10.64	0.00	4.55	1.33	0.97	80	117.20
1	5-6	35.70	10.64	0.00	4.55	1.22	0.97	80	117.20
1	6-7	16.30	10.64	0.00	2.94	1.03	0.97	80	117.20
1	7-8	16.30	10.64	0.00	2.94	0.83	0.97	80	117.20
1	8-9	10.60	10.64	0.00	1.77	0.54	0.97	80	117.20
1	9-10	10.60	10.64	0.00	1.77	0.38	0.97	80	117.20
1	10-11	16.30	10.64	0.00	2.94	0.24	0.97	80	117.20
1	11-12	16.30	10.64	0.00	2.94	0.15	0.97	80	117.20
1	12-13	10.60	10.64	0.00	1.77	0.09	0.97	80	117.20
1	13-14	10.60	10.64	0.00	1.77	0.05	0.97	80	117.20
1	14-15	16.30	10.64	0.00	2.94	0.03	0.97	80	117.20
1	15-16	16.30	10.64	0.00	2.94	0.02	0.97	80	117.20
1	16-17	10.60	10.64	0.00	1.77	0.01	0.97	80	117.20
1	17-18	10.60	10.64	0.00	1.77	0.01	0.97	80	117.20
1	18-19	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	19-20	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	20-21	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	21-22	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	22-23	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	23-24	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	24-25	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	25-26	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	26-27	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	27-28	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	28-29	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	29-30	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	30-31	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	31-32	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	32-33	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	33-34	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	34-35	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	35-36	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	36-37	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	37-38	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	38-39	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	39-40	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	40-41	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	41-42	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	42-43	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	43-44	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	44-45	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	45-46	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	46-47	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	47-48	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	48-49	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	49-50	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	50-51	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	51-52	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	52-53	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	53-54	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	54-55	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	55-56	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	56-57	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	57-58	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	58-59	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	59-60	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	60-61	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	61-62	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	62-63	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	63-64	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	64-65	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	65-66	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	66-67	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	67-68	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	68-69	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	69-70	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	70-71	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	71-72	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	72-73	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	73-74	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	74-75	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	75-76	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	76-77	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	77-78	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	78-79	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	79-80	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	80-81	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	81-82	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	82-83	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	83-84	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	84-85	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	85-86	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	86-87	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	87-88	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	88-89	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	89-90	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	90-91	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	91-92	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	92-93	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	93-94	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	94-95	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	95-96	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	96-97	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	97-98	10.60	10.64	0.00	1.77	0.00	0.97	80	117.20
1	98-99	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20
1	99-100	16.30	10.64	0.00	2.94	0.00	0.97	80	117.20

TABLE 3. ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

WATER CLOSING NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	117.103	100.030	17.073
2	0.00000	116.618	99.620	17.098
3	0.00000	115.297	99.310	15.987
4	0.00000	115.736	99.790	16.060
5	0.00000	116.790	99.930	16.140
6	1.90142	111.243	99.910	11.232
7	1.56733	111.143	99.909	11.231
8	0.37633	103.812	99.890	3.922



SCHEME 3

URWA, PART II, GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, ALLAHABAD

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SCHEME HAS 138 PIPES, 138 JUNCTIONS, 0 LOOP AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	0 1	100.0	25.0	130.0	61.912	0.643	1.261	6.432	114.00
2	1 2	340.0	25.0	130.0	26.534	1.004	0.047	8.478	114.00
3	2 3	750.0	20.0	130.0	16.534	0.231	0.540	1.338	114.00
4	3 4	140.0	20.0	130.0	15.120	0.424	0.455	1.246	114.00
5	4 5	150.0	30.0	140.0	15.375	0.378	0.483	1.408	114.00
6	5 6	900.0	20.0	130.0	12.068	0.676	0.495	1.882	114.00
7	6 7	120.0	20.0	130.0	12.021	0.703	0.510	1.036	114.00
8	7 8	140.0	20.0	130.0	10.389	2.493	0.520	1.521	114.00
9	8 9	140.0	20.0	130.0	10.388	0.448	0.509	1.525	114.00
10	9 10	450.0	20.0	130.0	14.972	1.568	0.530	1.757	114.00
11	10 11	250.0	20.0	130.0	10.533	1.504	0.570	1.300	114.00
12	11 12	250.0	20.0	130.0	10.532	1.622	0.615	1.490	114.00
13	12 13	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
14	13 14	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
15	14 15	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
16	15 16	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
17	16 17	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
18	17 18	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
19	18 19	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
20	19 20	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
21	20 21	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
22	21 22	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
23	22 23	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
24	23 24	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
25	24 25	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
26	25 26	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
27	26 27	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
28	27 28	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
29	28 29	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
30	29 30	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
31	30 31	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
32	31 32	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
33	32 33	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
34	33 34	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
35	34 35	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
36	35 36	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
37	36 37	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
38	37 38	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
39	38 39	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
40	39 40	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
41	40 41	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
42	41 42	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
43	42 43	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
44	43 44	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
45	44 45	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
46	45 46	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
47	46 47	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
48	47 48	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
49	48 49	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
50	49 50	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
51	50 51	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
52	51 52	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
53	52 53	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
54	53 54	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
55	54 55	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
56	55 56	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
57	56 57	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
58	57 58	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
59	58 59	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
60	59 60	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
61	60 61	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
62	61 62	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
63	62 63	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
64	63 64	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
65	64 65	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
66	65 66	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
67	66 67	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
68	67 68	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
69	68 69	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
70	69 70	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
71	70 71	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
72	71 72	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
73	72 73	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
74	73 74	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
75	74 75	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
76	75 76	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
77	76 77	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
78	77 78	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
79	78 79	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
80	79 80	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
81	80 81	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
82	81 82	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
83	82 83	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
84	83 84	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
85	84 85	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
86	85 86	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
87	86 87	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
88	87 88	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
89	88 89	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
90	89 90	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
91	90 91	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
92	91 92	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
93	92 93	140.0	20.0	130.0	10.532	1.622	0.604	1.322	114.00
94	93 94	140.0	20.0						



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THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

CONCUSSION NO.	DEMAND (CLT/1/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.0000	113.357	109.200	13.16
2	0.0250	110.474	109.870	10.60
3	0.0000	112.353	100.410	11.94
4	0.0436	109.090	100.180	9.82
5	0.0000	112.120	100.240	11.88
6	0.4810	109.755	99.870	9.88
7	0.11970	110.817	99.400	11.39
8	0.12500	110.332	99.400	10.93
9	0.25000	110.535	99.400	11.14
10	0.12500	110.143	98.760	11.31
11	0.37500	110.767	98.760	12.00
12	0.25000	109.797	98.760	11.03







102	0.25670	106.540	88.707	07
103	0.20000	107.547	88.707	07
104	0.13890	105.417	88.707	07
105	0.13890	106.218	88.707	07
106	0.13890	105.195	88.707	07
107	0.17500	105.775	88.707	07
108	0.14861	104.202	88.707	07
109	0.09625	102.495	88.707	07
110	0.19440	104.202	88.707	07
111	0.00000	103.309	88.707	07
112	0.00000	103.309	88.707	07
113	0.00000	103.309	88.707	07
114	0.00000	103.309	88.707	07
115	0.00000	103.309	88.707	07
116	0.00000	103.309	88.707	07
117	0.00000	103.309	88.707	07
118	0.00000	103.309	88.707	07
119	0.00000	103.309	88.707	07
120	0.00000	103.309	88.707	07
121	0.00000	103.309	88.707	07
122	0.00000	103.309	88.707	07
123	0.00000	103.309	88.707	07
124	0.00000	103.309	88.707	07
125	0.00000	103.309	88.707	07
126	0.00000	103.309	88.707	07
127	0.00000	103.309	88.707	07
128	0.00000	103.309	88.707	07
129	0.00000	103.309	88.707	07
130	0.00000	103.309	88.707	07
131	0.00000	103.309	88.707	07
132	0.00000	103.309	88.707	07
133	0.00000	103.309	88.707	07
134	0.00000	103.309	88.707	07
135	0.00000	103.309	88.707	07
136	0.00000	103.309	88.707	07
137	0.00000	103.309	88.707	07
138	0.00000	103.309	88.707	07
139	0.00000	103.309	88.707	07
140	0.00000	103.309	88.707	07
141	0.00000	103.309	88.707	07
142	0.00000	103.309	88.707	07
143	0.00000	103.309	88.707	07
144	0.00000	103.309	88.707	07
145	0.00000	103.309	88.707	07
146	0.00000	103.309	88.707	07
147	0.00000	103.309	88.707	07
148	0.00000	103.309	88.707	07
149	0.00000	103.309	88.707	07
150	0.00000	103.309	88.707	07

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CM)	ROUGHNESS	RATE (RS)	TOTAL LENGTH (METERS)	COST (RS)
25.00	130.00	112.00	850.00	95200.00
20.00	130.00	112.00	850.00	71196.00
15.00	130.00	112.00	850.00	119301.00
10.00	130.00	112.00	850.00	170568.00
7.50	130.00	112.00	850.00	194944.00
5.00	130.00	112.00	850.00	29159.00
3.00	130.00	112.00	850.00	30202.00
2.00	130.00	112.00	850.00	13120.00
TOTAL COST=				1500587.90



THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CM'S)	ROUGHNESS
66	66	570.0	8.0	140.0
67	77	200.0	12.5	140.0
77	78	150.0	12.5	140.0
78	82	150.0	12.5	140.0
82	84	450.0	12.5	140.0
84	87	160.0	12.5	140.0
87	90	75.0	12.5	140.0
90	99	980.0	3.0	140.0
99	116	140.0	3.2	140.0
116	117	100.0	3.2	140.0
117	120	140.0	3.2	140.0
120	124	150.0	3.2	140.0
124	151	9220.0	3.2	140.0
151	153	680.0	3.2	140.0
153	168	1500.0	3.2	140.0
168	169	520.0	3.0	140.0

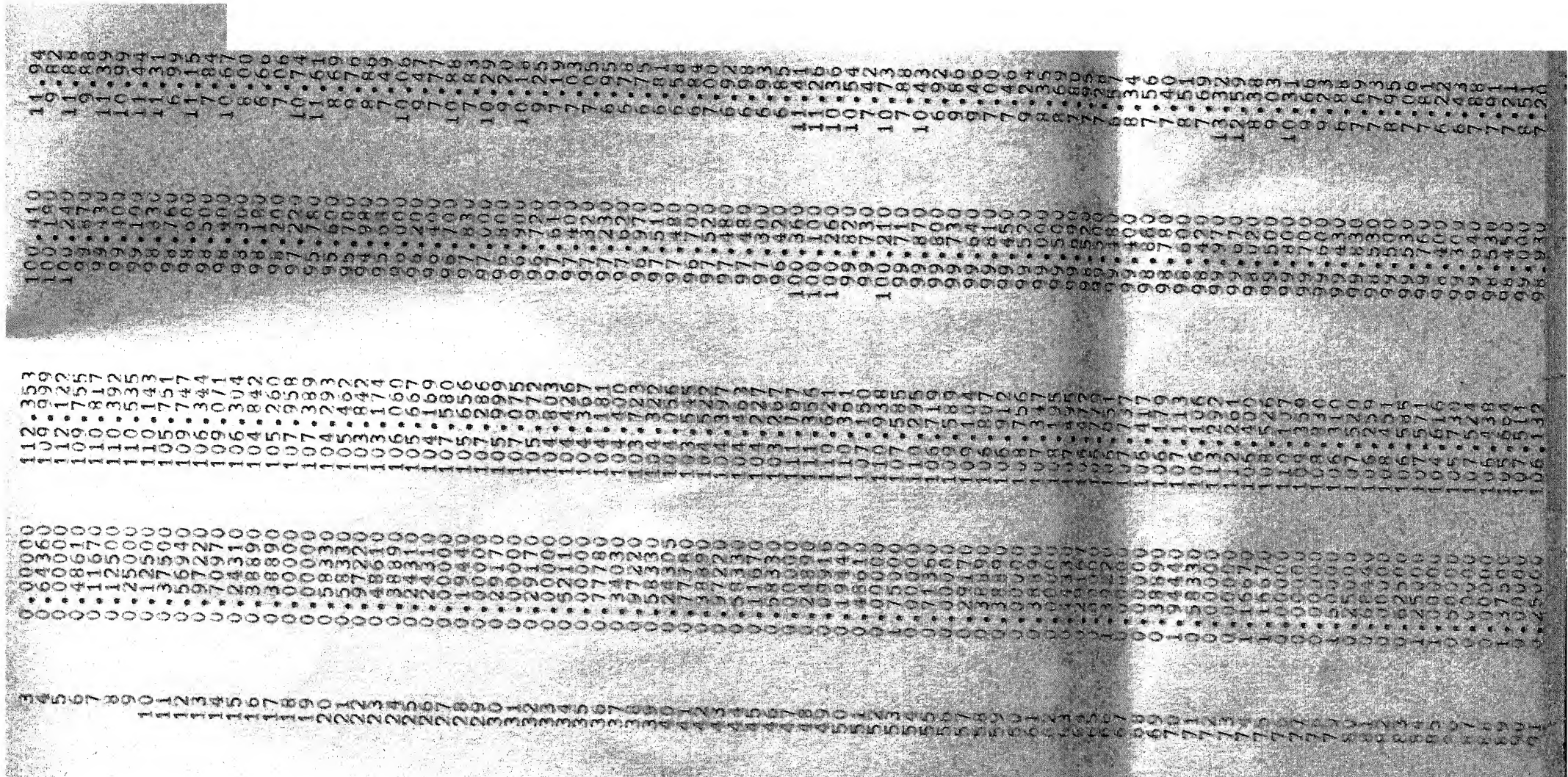
THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAM (CM)	METER	ROUGHNESS	FLOW RATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HD/1000	FIXED GRADE (METERS)
1	1-2	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
2	1-3	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
3	1-4	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
4	1-5	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
5	1-6	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
6	1-7	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
7	1-8	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
8	1-9	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
9	1-10	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
10	1-11	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
11	1-12	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
12	1-13	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
13	1-14	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
14	1-15	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
15	1-16	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
16	1-17	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
17	1-18	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
18	1-19	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
19	1-20	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
20	1-21	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
21	1-22	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
22	1-23	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
23	1-24	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
24	1-25	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
25	1-26	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
26	1-27	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
27	1-28	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
28	1-29	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
29	1-30	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
30	1-31	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
31	1-32	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
32	1-33	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
33	1-34	100.00	25.00	0.00	1.00	0.00	0.00	1.00	1.00	114.00
34	1-35									











104.	804	97.	766
108.	723	99.	900
105.	583	96.	860
108.	521	99.	800
107.	834	96.	657
104.	150	97.	830
106.	557	94.	680
107.	651	94.	730
106.	695	98.	610
106.	070	98.	550
106.	159	98.	570
107.	540	96.	750
105.	417	97.	660
106.	211	98.	460
105.	198	97.	450
104.	775	97.	830
104.	202	97.	500
103.	570	97.	600
104.	114	97.	440
103.	307	96.	380
109.	731	100.	300
108.	508	99.	600
106.	934	98.	456
108.	286	98.	360
107.	743	100.	300
106.	134	99.	720
107.	611	99.	910
107.	784	99.	870
106.	853	99.	730
106.	337	99.	820
105.	377	98.	400
104.	438	97.	700
104.	439	97.	870
104.	497	97.	500
104.	892	97.	480
107.	566	97.	600
106.	526	97.	730
107.	551	98.	760
106.	591	98.	900
106.	710	99.	000
106.	522	98.	800
106.	288	98.	860
103.	958	97.	800

QUANTITY	POUGHNESS	RATE (/GHS)	TOTAL LENGTH (METERS)	COST (RS)
130.00	130.00	112.00	850.00	95200.00
130.00	130.00	113.00	850.00	95350.00
130.00	130.00	114.00	850.00	95500.00
130.00	130.00	115.00	850.00	95650.00
130.00	130.00	116.00	850.00	95800.00
130.00	130.00	117.00	850.00	95950.00
130.00	130.00	118.00	850.00	96100.00
130.00	130.00	119.00	850.00	96250.00
130.00	130.00	120.00	850.00	96400.00
130.00	130.00	121.00	850.00	96550.00
130.00	130.00	122.00	850.00	96700.00
130.00	130.00	123.00	850.00	96850.00
130.00	130.00	124.00	850.00	97000.00
130.00	130.00	125.00	850.00	97150.00
130.00	130.00	126.00	850.00	97300.00
130.00	130.00	127.00	850.00	97450.00
130.00	130.00	128.00	850.00	97600.00
130.00	130.00	129.00	850.00	97750.00
130.00	130.00	130.00	850.00	97900.00
130.00	130.00	131.00	850.00	98050.00
130.00	130.00	132.00	850.00	98200.00
130.00	130.00	133.00	850.00	98350.00
130.00	130.00	134.00	850.00	98500.00
130.00	130.00	135.00	850.00	98650.00
130.00	130.00	136.00	850.00	98800.00
130.00	130.00	137.00	850.00	98950.00
130.00	130.00	138.00	850.00	99100.00
130.00	130.00	139.00	850.00	99250.00
130.00	130.00	140.00	850.00	99400.00
130.00	130.00	141.00	850.00	99550.00
130.00	130.00	142.00	850.00	99700.00
130.00	130.00	143.00	850.00	99850.00
130.00	130.00	144.00	850.00	100000.00
130.00	130.00	145.00	850.00	100150.00
130.00	130.00	146.00	850.00	100300.00
130.00	130.00	147.00	850.00	100450.00
130.00	130.00	148.00	850.00	100600.00
130.00	130.00	149.00	850.00	100750.00
130.00	130.00	150.00	850.00	100900.00
130.00	130.00	151.00	850.00	101050.00
130.00	130.00	152.00	850.00	101200.00
130.00	130.00	153.00	850.00	101350.00
130.00	130.00	154.00	850.00	101500.00
130.00	130.00	155.00	850.00	101650.00
130.00	130.00	156.00	850.00	101800.00
130.00	130.00	157.00	850.00	101950.00
130.00	130.00	158.00	850.00	102100.00
130.00	130.00	159.00	850.00	102250.00
130.00	130.00	160.00	850.00	102400.00
130.00	130.00	161.00	850.00	102550.00
130.00	130.00	162.00	850.00	102700.00
130.00	130.00	163.00	850.00	102850.00
130.00	130.00	164.00	850.00	103000.00
130.00	130.00	165.00	850.00	103150.00
130.00	130.00	166.00	850.00	103300.00
130.00	130.00	167.00	850.00	103450.00
130.00	130.00	168.00	850.00	103600.00
130.00	130.00	169.00	850.00	103750.0



SCHEME 4

MALASA GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, KANPUR

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SYSTEM HAS 298 PIPES, 242 JUNCTIONS, 56 LOOPS AND 1 FIXED GRADE NODE

THE FOLLOWING RESULTS ARE OBTAINED AFTER 5 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CM)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADES (METERS)
1	1-2	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
2	2-3	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
3	3-4	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
4	4-5	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
5	5-6	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
6	6-7	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
7	7-8	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
8	8-9	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
9	9-10	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
10	10-11	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
11	11-12	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
12	12-13	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
13	13-14	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
14	14-15	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
15	15-16	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
16	16-17	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
17	17-18	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
18	18-19	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
19	19-20	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
20	20-21	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
21	21-22	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
22	22-23	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
23	23-24	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
24	24-25	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
25	25-26	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
26	26-27	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
27	27-28	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
28	28-29	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
29	29-30	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
30	30-31	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
31	31-32	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
32	32-33	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
33	33-34	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
34	34-35	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
35	35-36	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
36	36-37	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
37	37-38	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
38	38-39	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
39	39-40	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
40	40-41	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
41	41-42	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
42	42-43	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
43	43-44	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
44	44-45	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
45	45-46	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
46	46-47	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
47	47-48	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
48	48-49	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
49	49-50	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
50	50-51	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
51	51-52	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
52	52-53	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
53	53-54	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
54	54-55	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
55	55-56	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
56	56-57	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
57	57-58	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
58	58-59	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
59	59-60	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
60	60-61	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
61	61-62	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
62	62-63	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
63	63-64	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
64	64-65	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
65	65-66	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
66	66-67	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
67	67-68	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
68	68-69	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
69	69-70	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
70	70-71	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
71	71-72	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
72	72-73	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
73	73-74	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
74	74-75	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
75	75-76	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
76	76-77	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
77	77-78	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
78	78-79	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
79	79-80	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
80	80-81	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
81	81-82	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
82	82-83	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
83	83-84	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
84	84-85	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
85	85-86	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
86	86-87	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
87	87-88	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
88	88-89	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
89	89-90	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
90	90-91	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
91	91-92	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
92	92-93	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
93	93-94	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
94	94-95	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
95	95-96	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
96	96-97	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
97	97-98	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
98	98-99	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
99	99-100	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81
100	100-101	10.0	10.0	0.0	45.0	0.15	0.0	5.5	143.81







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134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644
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UNIT NO.	REWARD (1/SEC)	GRADE	LINE (1/SEC)	ELEVATION	PRESSURE (1/SEC)
1	0.0000	135.0	177.4	123.770	110.00
2	0.0000	135.0	177.4	123.770	110.00
3	0.0000	135.0	177.4	123.770	110.00
4	0.0000	135.0	177.4	123.770	110.00
5	0.0000	135.0	177.4	123.770	110.00
6	0.0000	135.0	177.4	123.770	110.00
7	0.0000	135.0	177.4	123.770	110.00
8	0.0000	135.0	177.4	123.770	110.00
9	0.0000	135.0	177.4	123.770	110.00
10	0.0000	135.0	177.4	123.770	110.00
11	0.0000	135.0	177.4	123.770	110.00
12	0.0000	135.0	177.4	123.770	110.00
13	0.0000	135.0	177.4	123.770	110.00
14	0.0000	135.0	177.4	123.770	110.00
15	0.0000	135.0	177.4	123.770	110.00
16	0.0000	135.0	177.4	123.770	110.00
17	0.0000	135.0	177.4	123.770	110.00
18	0.0000	135.0	177.4	123.770	110.00
19	0.0000	135.0	177.4	123.770	110.00
20	0.0000	135.0	177.4	123.770	110.00
21	0.0000	135.0	177.4	123.770	110.00
22	0.0000	135.0	177.4	123.770	110.00
23	0.0000	135.0	177.4	123.770	110.00
24	0.0000	135.0	177.4	123.770	110.00
25	0.0000	135.0	177.4	123.770	110.00
26	0.0000	135.0	177.4	123.770	110.00
27	0.0000	135.0	177.4	123.770	110.00
28	0.0000	135.0	177.4	123.770	110.00
29	0.0000	135.0	177.4	123.770	110.00
30	0.0000	135.0	177.4	123.770	110.00



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DIAMETER (CM)	ROUGHNESS	RATE (/HRS)	TOTAL LENGTH (METERS)	COST (LRS)
20.00	130.0	55.00	100.00	5500.00
20.00	130.0	40.00	9425.00	377000.00
20.00	130.0	30.00	68022.00	2040660.00
20.00	130.0	25.00	66022.00	1650550.00
20.00	130.0	17.00	2528.00	42976.00
20.00	130.0	12.70	348.00	4419.60
20.00	130.0	7.70	4469.00	33320.00
20.00	130.0	7.00	32170.00	225190.00
20.00	130.0	4.15	2860.00	11489.00
20.00	130.0	4.10	2865.00	11869.50
				913214.70



THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMG)	ROUGHNESS
82	72	80.0	3.0	140.0
77	75	275.0	3.2	140.0
1029	88	100.0	3.5	140.0
1109	94	120.0	3.2	140.0
1122	96	115.0	3.2	140.0
1135	105	170.0	3.2	140.0
1152	108	15.0	3.0	140.0
1167	113	65.0	3.2	140.0
1178	113	110.0	3.2	140.0
1187	113	60.0	3.2	140.0
1199	114	60.0	3.2	140.0
1212	115	86.0	3.2	140.0
1224	115	152.0	3.5	140.0
1235	117	180.0	3.5	140.0
1246	117	145.0	3.5	140.0
1257	118	40.0	3.0	140.0
1269	119	40.0	3.0	140.0
1281	122	125.0	3.0	140.0
1293	122	135.0	3.0	140.0
1305	123	150.0	3.0	140.0
1317	123	180.0	3.0	140.0
1329	123	130.0	3.0	140.0
1341	124	120.0	3.0	140.0
1353	124	150.0	3.0	140.0
1365	125	120.0	3.0	140.0
1377	125	150.0	3.0	140.0
1389	126	120.0	3.0	140.0
1401	126	150.0	3.0	140.0
1413	127	120.0	3.0	140.0
1425	127	150.0	3.0	140.0
1437	128	120.0	3.0	140.0
1449	128	150.0	3.0	140.0
1461	129	120.0	3.0	140.0
1473	129	150.0	3.0	140.0
1485	130	120.0	3.0	140.0
1497	130	150.0	3.0	140.0
1509	131	120.0	3.0	140.0
1521	131	150.0	3.0	140.0
1533	132	120.0	3.0	140.0
1545	132	150.0	3.0	140.0
1557	133	120.0	3.0	140.0
1569	133	150.0	3.0	140.0
1581	134	120.0	3.0	140.0
1593	134	150.0	3.0	140.0
1605	135	120.0	3.0	140.0
1617	135	150.0	3.0	140.0
1629	136	120.0	3.0	140.0
1641	136	150.0	3.0	140.0
1653	137	120.0	3.0	140.0
1665	137	150.0	3.0	140.0
1677	138	120.0	3.0	140.0
1689	138	150.0	3.0	140.0
1701	139	120.0	3.0	140.0
1713	139	150.0	3.0	140.0
1725	140	120.0	3.0	140.0
1737	140	150.0	3.0	140.0
1749	141	120.0	3.0	140.0
1761	141	150.0	3.0	140.0
1773	142	120.0	3.0	140.0
1785	142	150.0	3.0	140.0
1797	143	120.0	3.0	140.0
1809	143	150.0	3.0	140.0
1821	144	120.0	3.0	140.0
1833	144	150.0	3.0	140.0
1845	145	120.0	3.0	140.0
1857	145	150.0	3.0	140.0
1869	146	120.0	3.0	140.0
1881	146	150.0	3.0	140.0
1893	147	120.0	3.0	140.0
1905	147	150.0	3.0	140.0
1917	148	120.0	3.0	140.0
1929	148	150.0	3.0	140.0
1941	149	120.0	3.0	140.0
1953	149	150.0	3.0	140.0
1965	150	120.0	3.0	140.0
1977	150	150.0	3.0	140.0
1989	151	120.0	3.0	140.0
2001	151	150.0	3.0	140.0
2013	152	120.0	3.0	140.0
2025	152	150.0	3.0	140.0
2037	153	120.0	3.0	140.0
2049	153	150.0	3.0	140.0
2061	154	120.0	3.0	140.0
2073	154	150.0	3.0	140.0
2085	155	120.0	3.0	140.0
2097	155	150.0	3.0	140.0
2109	156	120.0	3.0	140.0
2121	156	150.0	3.0	140.0
2133	157	120.0	3.0	140.0
2145	157	150.0	3.0	140.0
2157	158	120.0	3.0	140.0
2169	158	150.0	3.0	140.0
2181	159	120.0	3.0	140.0
2193	159	150.0	3.0	140.0
2205	160	120.0	3.0	140.0
2217	160	150.0	3.0	140.0
2229	161	120.0	3.0	140.0
2241	161	150.0	3.0	140.0
2253	162	120.0	3.0	140.0
2265	162	150.0	3.0	140.0
2277	163	120.0	3.0	140.0
2289	163	150.0	3.0	140.0
2301	164	120.0	3.0	140.0
2313	164	150.0	3.0	140.0
2325	165	120.0	3.0	140.0
2337	165	150.0	3.0	140.0
2349	166	120.0	3.0	140.0
2361	166	150.0	3.0	140.0
2373	167	120.0	3.0	140.0
2385	167	150.0	3.0	140.0
2397	168	120.0	3.0	140.0
2409	168	150.0	3.0	140.0
2421	169	120.0	3.0	140.0
2433	169	150.0	3.0	140.0
2445	170	120.0	3.0	140.0
2457	170	150.0	3.0	140.0
2469	171	120.0	3.0	140.0
2481	171	150.0	3.0	140.0
2493	172	120.0	3.0	140.0
2505	172	150.0	3.0	140.0
2517	173	120.0	3.0	140.0
2529	173	150.0	3.0	140.0
2541	174	120.0	3.0	140.0
2553	174	150.0	3.0	140.0
2565	175	120.0	3.0	140.0
2577	175	150.0	3.0	140.0
2589	176	120.0	3.0	140.0
2601	176	150.0	3.0	140.0
2613	177	120.0	3.0	140.0
2625	177	150.0	3.0	140.0
2637	178	120.0	3.0	140.0
2649	178	150.0	3.0	140.0
2661	179	120.0	3.0	140.0
2673	179	150.0	3.0	140.0
2685	180	120.0	3.0	140.0
2697	180	150.0	3.0	140.0
2709	181	120.0	3.0	140.0
2721	181	150.0	3.0	140.0
2733	182	120.0	3.0	140.0
2745	182	150.0	3.0	140.0
2757	183	120.0	3.0	140.0
2769	183	150.0	3.0	140.0
2781	184	120.0	3.0	140.0
2793	184	150.0	3.0	140.0
2805	185	120.0	3.0	140.0
2817	185	150.0	3.0	140.0
2829	186	120.0	3.0	140.0
2841	186	150.0	3.0	140.0
2853	187	120.0	3.0	140.0
2865	187	150.0	3.0	140.0
2877	188	120.0	3.0	140.0
2889	188	150.0	3.0	140.0
2901	189	120.0	3.0	140.0
2913	189	150.0	3.0	140.0
2925	190	120.0	3.0	140.0
2937	190	150.0	3.0	140.0
2949	191	120.0	3.0	140.0
2961	191	150.0	3.0	140.0
2973	192	120.0	3.0	140.0
2985	192	150.0	3.0	140.0
2997	193	120.0	3.0	140.0
3009	193	150.0	3.0	140.0
3021	194	120.0	3.0	140.0
3033	194	150.0	3.0	140.0
3045	195	120.0	3.0	140.0
3057	195	150.0	3.0	140.0
3069	196	120.0	3.0	140.0
3081	196	150.0	3.0	140.0
3093	197	120.0	3.0	140.0
3105	197	150.0	3.0	140.0
3117	198	120.0	3.0	140.0
3129	198	150.0	3.0	140.0
3141	199	120.0	3.0	140.0
3153	199	150.0	3.0	140.0
3165	200	120.0	3.0	140.0
3177	200	150.0	3.0	140.0
3189	201	120.0	3.0	140.0
3201	201	150.0	3.0	140.0
3213	202	120.0	3.0	140.0
3225	202	150.0	3.0	140.0
3237	203	120.0	3.0	140.0
3249	203	150.0	3.0	140.0
3261	204	120.0	3.0	140.0
3273	204	150.0	3.0	140.0
3285	205	120.0	3.0	140.0
3297	205	150.0	3.0	140.0
3309	206	120.0	3.0	140.0
3321	206	150.0	3.0	140.0
3333	207	120.0	3.0	140.0
3345	207	150.0	3.0	140.0
3357	208	120.0	3.0	140.0
3369	208	150.0	3.0	140.0
3381	209	120.0	3.0	140.0
3393	209	150.0	3.0	140.0
3405	210	120.0	3.0	140.0
3417	210	150.0	3.0	140.0
3429	211	120.0	3.0	140.0
3441	211	150.0	3.0	140.0
3453	212	120.0	3.0	140.0
3465	212	150.0	3.0	140.0
3477	213	120.0	3.0	140.0
3489	213	150.0	3.0	140.0
3501	214	120.0	3.0	140.0
3513	214	150.0	3.0	140.0
3525	215	120.0	3.0	140.0
3537	215	150.0	3.0	140.0
3549	216	120.0	3.0	140.0
3561	216	150.0	3.0	140.0
3573	217	120.0	3.0	140.0
3585	217	150.0	3.0	140.0
3597	218	120.0	3.0	140.0
3609	218	150.0	3.0	140.0
3621	219	120.0	3.0	140.0
3633	219	150.0	3.0	140.0
3645	220	120.0	3.0	140.0
3657	220	150.0	3.0	140.0
3669	221	120.0	3.0	140.0
3681	221	150.0	3.0	140.0
3693	222	120.0	3.0	140.0
3705	222	150.0	3.0	140.0
3717	223	120.0	3.0	140.0
3729	223	150.0	3.0	140.0
3741	224	120.0	3.0	140.0
3753	224	150.0	3.0	140.0
3765	225	120.0	3.0	140.0
3777	225	150.0	3.0	140.0
3789	226	120.0	3.0	140.0
3801	226	150.0	3.0	140.0
3813	227	120.0	3.0	140.0
3825	227	150.0	3.0	140.0
3837	228	120.0	3.0	140.0
3849	228	150.0	3.0	140.0
3861	229	120.0	3.0	140.0
3873	229	150.0	3.0	140.0
3885	230	120.0	3.0	140.0
3897	230	150.0	3.0	140.0
3909	231	120.0	3.0	140.0
3921	231	150.0	3.0	140.0
3933	232	120.0	3.0	140.0
3945	232	150.0	3.0	140.0
3957	233	120.0	3.0	140.0
3969	233	150.0	3.0	140.0
3981	234	120.0	3.0	140.0
3993	234	150.0	3.0	140.0
4005	235	120.0	3.0	140.0
4017	235	150.0</		















JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	DATE
1	0.00000	143.774	123.770	1953
2	0.21390	143.616	122.890	1953
3	0.33060	143.330	122.240	1953
4	0.33060	143.195	122.710	1953
5	0.15000	142.481	125.150	1953
6	0.00000	136.935	125.720	1953
7	0.19440	134.401	125.020	1953
8	0.33060	134.227	125.290	1953
9	0.33060	134.319	124.310	1953
10	0.15330	134.037	124.410	1953
11	0.33060	133.947	124.330	1953
12	0.17780	133.333	124.530	1953
13	0.16170	133.730	124.570	1953
14	0.33060	133.255	127.810	1953
15	0.13610	133.715	125.340	1953
16	0.33060	131.503	126.950	1953
17	0.00000	141.553	126.000	1953
18	0.00000	141.553	126.000	1953
19	0.00000	135.314	127.870	1953
20	0.00000	135.060	128.050	1953
21	0.00000	133.225	126.550	1953
22	0.15000	135.730	126.240	1953
23	0.00000	135.710	126.530	1953
24	0.00000	134.335	126.750	1953
25	0.00000	134.037	125.820	1953
26	0.00000	141.034	128.050	1953
27	0.00000	141.034	128.050	1953
28	0.00000	141.034	128.050	1953
29	0.00000	141.034	128.050	1953
30	0.00000	141.034	128.050	1953
31	0.00000	141.034	128.050	1953
32	0.00000	141.034	128.050	1953
33	0.00000	141.034	128.050	1953
34	0.00000	141.034	128.050	1953
35	0.00000	141.034	128.050	1953
36	0.00000	141.034	128.050	1953
37	0.00000	141.034	128.050	1953
38	0.00000	141.034	128.050	1953
39	0.00000	141.034	128.050	1953
40	0.00000	141.034	128.050	1953
41	0.00000	141.034	128.050	1953
42	0.00000	141.034	128.050	1953
43	0.00000	141.034	128.050	1953
44	0.00000	141.034	128.050	1953
45	0.00000	141.034	128.050	1953
46	0.00000	141.034	128.050	1953
47	0.00000	141.034	128.050	1953
48	0.00000	141.034	128.050	1953
49	0.00000	141.034	128.050	1953
50	0.00000	141.034	128.050	1953
51	0.00000	141.034	128.050	1953
52	0.00000	141.034	128.050	1953
53	0.00000	141.034	128.050	1953
54	0.00000	141.034	128.050	1953
55	0.00000	141.034	128.050	1953
56	0.00000	141.034	128.050	1953
57	0.00000	141.034	128.050	1953
58	0.00000	141.034	128.050	1953
59	0.00000	141.034	128.050	1953
60	0.00000	141.034	128.050	1953
61	0.00000	141.034	128.050	1953
62	0.00000	141.034	128.050	1953
63	0.00000	141.034	128.050	1953
64	0.00000	141.034	128.050	1953
65	0.00000	141.034	128.050	1953
66	0.00000	141.034	128.050	1953
67	0.00000	141.034	128.050	1953
68	0.00000	141.034	128.050	1953
69	0.00000	141.034	128.050	1953
70	0.00000	141.034	128.050	1953











[illegible]



SCHEME 5

PAJPUR GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, KANPUR

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SYSTEM HAS 86 PIPES, 60 JUNCTIONS, 20 LOOPS AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 6 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1-2	100.00	20.00	130.00	22.389	0.261	0.713	2.22	899
2	1-3	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
3	1-4	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
4	1-5	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
5	1-6	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
6	1-7	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
7	1-8	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
8	1-9	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
9	1-10	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
10	1-11	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
11	1-12	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
12	1-13	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
13	1-14	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
14	1-15	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
15	1-16	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
16	1-17	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
17	1-18	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
18	1-19	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
19	1-20	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
20	1-21	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
21	1-22	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
22	1-23	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
23	1-24	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
24	1-25	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
25	1-26	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
26	1-27	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
27	1-28	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
28	1-29	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
29	1-30	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
30	1-31	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
31	1-32	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
32	1-33	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
33	1-34	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
34	1-35	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
35	1-36	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
36	1-37	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
37	1-38	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
38	1-39	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
39	1-40	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
40	1-41	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
41	1-42	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
42	1-43	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
43	1-44	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
44	1-45	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
45	1-46	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
46	1-47	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
47	1-48	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
48	1-49	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
49	1-50	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
50	1-51	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
51	1-52	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
52	1-53	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
53	1-54	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
54	1-55	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
55	1-56	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
56	1-57	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
57	1-58	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
58	1-59	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
59	1-60	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
60	1-61	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
61	1-62	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
62	1-63	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
63	1-64	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
64	1-65	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
65	1-66	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
66	1-67	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
67	1-68	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
68	1-69	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
69	1-70	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
70	1-71	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
71	1-72	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
72	1-73	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
73	1-74	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
74	1-75	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
75	1-76	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
76	1-77	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
77	1-78	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
78	1-79	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
79	1-80	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
80	1-81	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
81	1-82	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
82	1-83	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
83	1-84	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
84	1-85	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
85	1-86	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899
86	1-87	150.00	20.00	130.00	22.444	0.367	0.751	2.22	899







THE FOLLOWING IS THE COST STATEMENT OF THE SCHELF

DIAMETER (CMS)	ROUGHNESS	RATE (/M(RS))	TOTAL LENGTH (M(RS))	COST (RS)
20.00	130.0	40.00	310.0	12400.00
15.5	130.0	28.00	1477.0	41374.16
12.0	130.0	24.00	235.0	5640.00
10.0	130.0	19.00	1457.0	27683.00
8.5	130.0	16.50	3465.0	57172.50
5.0	130.0	10.50	1265.0	13282.50
3.2	140.0	7.60	1315.0	9994.00
	140.0	5.60	1495.0	8372.00
	140.0	4.10	465.0	1906.50
TOTAL COST				177924.66



THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	MODE	NUMBERS	LENGTH (METERS)	DIAMETER (CM)	ROUGHNESS
71	47	115949	180.00	2.50	140.00
72	47	115949	170.00	2.50	140.00
73	47	115949	160.00	2.50	140.00
74	47	115949	150.00	2.50	140.00
75	47	115949	140.00	2.50	140.00
76	47	115949	130.00	2.50	140.00
77	47	115949	120.00	2.50	140.00
78	47	115949	110.00	2.50	140.00
79	47	115949	100.00	2.50	140.00
80	47	115949	90.00	2.50	140.00
81	47	115949	80.00	2.50	140.00
82	47	115949	70.00	2.50	140.00
83	47	115949	60.00	2.50	140.00
84	47	115949	50.00	2.50	140.00
85	47	115949	40.00	2.50	140.00
86	47	115949	30.00	2.50	140.00
87	47	115949	20.00	2.50	140.00
88	47	115949	10.00	2.50	140.00
89	47	115949	0.00	2.50	140.00
90	47	115949	0.00	2.50	140.00
91	47	115949	0.00	2.50	140.00
92	47	115949	0.00	2.50	140.00
93	47	115949	0.00	2.50	140.00
94	47	115949	0.00	2.50	140.00
95	47	115949	0.00	2.50	140.00
96	47	115949	0.00	2.50	140.00
97	47	115949	0.00	2.50	140.00
98	47	115949	0.00	2.50	140.00
99	47	115949	0.00	2.50	140.00
100	47	115949	0.00	2.50	140.00

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	MODE	NUMBERS	LENGTH (METERS)	DIAMETER (CM)	ROUGHNESS	FLOW RATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
71	47	115949	180.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
72	47	115949	170.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
73	47	115949	160.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
74	47	115949	150.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
75	47	115949	140.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
76	47	115949	130.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
77	47	115949	120.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
78	47	115949	110.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
79	47	115949	100.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
80	47	115949	90.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
81	47	115949	80.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
82	47	115949	70.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
83	47	115949	60.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
84	47	115949	50.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
85	47	115949	40.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
86	47	115949	30.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
87	47	115949	20.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
88	47	115949	10.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
89	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
90	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
91	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
92	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
93	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
94	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
95	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
96	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
97	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
98	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
99	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00
100	47	115949	0.00	2.50	140.00	22.444	0.361	0.713	2.5	111.00



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THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

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43	0.77778	106.710	97.910	80.87
44	0.77778	106.665	97.900	88.82
45	1.74999	106.011	99.365	87.82
46	1.16669	106.171	99.350	87.82
47	1.38889	106.374	97.325	87.82
48	0.19444	106.525	97.325	87.82
49	0.38889	106.646	97.325	87.82
50	0.77778	106.706	96.265	87.82
51	0.58000	106.672	96.265	87.82
52	0.00000	107.128	97.575	87.82
53	0.58333	107.120	97.575	87.82
54	0.48611	106.793	97.575	87.82
55	0.09722	106.879	97.295	87.82
56	0.09722	106.949	97.295	87.82
57	0.19444	107.071	96.985	87.82
58	0.19444	106.666	96.985	87.82
59	0.25000	107.540	97.803	87.82

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEM

DIAMETER (CM)	ROUGHNESS	RATE (/MRS)	TOTAL LENGTH (METERS)	COST (MRS)
20.00	130.00	40.00	240.00	9600.00
15.00	130.00	28.00	1347.00	37516.00
12.50	130.00	28.00	1735.00	48580.00
10.00	130.00	16.50	192.00	3168.00
5.00	140.00	7.50	2535.00	19012.50
3.00	140.00	5.10	1480.00	7548.00
			1060.00	4346.00
				156582.76